



# Structural Calculations

for

Spring Run Subdivision - Building 11  
The Boulder 4-Plex

Eagle Mountain, Utah

submitted to:

ARCFLO

Solutions you can build on for over 70 years

contact:

Jeff Turville, PE

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Riverdale, Utah 84405  
801.621.3100



## Notice

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November 2023  
4899-A97

## Project Information

Project Name: Spring Run Subdivision - Building 11 - The Boulder 4-Plex  
 Project Location: Eagle Mountain, Utah

## Design Criteria

Governing Building Code: 2021 IBC

Construction Type: Wood Bearing Wall

Wind Zone and Exposure: 115mph. (3 sec. gust), Exp C

Seismic Design Category: D

Soil Site Class: D

Spectral Accelerations:  $S_S = 1.139g$     $S_{DS} = 0.793g$   
 $S_1 = 0.385g$     $S_{D1} = 0.418g$

Design Loads:

- Roof Dead Load = 15 psf
- Ground Snow Load = 45 psf
- Roof Snow Load = 35 psf
- Floor Dead Load = 15 psf
- Floor Live Load = 40 psf

## Construction Materials

### Concrete 28-Day Compressive Strength

Foundations:  $f'_c = 3000$  psi (2500 psi design)

Exterior Slabs on Grade:  $f'_c = 4000$  psi

Reinforcing Grade: ASTM A615 Grade 60

Structural Steel          ASTM A992 ( $f_y = 50000$  psi)

### Wood

Sawn Lumber:

DF#2 or better       $F_b=875$  psi       $F_v=95$  psi       $E=1.6 \cdot 10^6$  psi

Laminated Veneer Lumber:

Microllam®           $F_b=2600$  psi       $F_v=285$  psi       $E=1.9 \cdot 10^6$  psi

Parallel Strand Lumber:

Parallam®           $F_b=2900$  psi       $F_v=290$  psi       $E=2.0 \cdot 10^6$  psi

Glu-Laminated Beams:

24F-V4 DF/DF       $F_b=2400$  psi       $F_v=195$  psi       $E=1.8 \cdot 10^6$  psi

Roof Sheathing          15/32" OSB

Floor Sheathing          3/4" OSB

Wall Sheathing          7/16" OSB

## Soil Criteria

Geotechnical Consultant: None

Report Number: N/A

Bearing Pressure: 1500 psf (Assumed)

Min. Bearing Depth: 30" to bottom of footing  
*(Contractor/Owner to verify proper bearing conditions are provided)*

# USGS Design Maps Summary Report

## User-Specified Input

Building Code Reference Document ASCE7-16

Site Coordinates 40.38966°N, 111.8474°W

Site Soil Classification Site Class D – “Stiff Soil”

Risk Category I/II/III

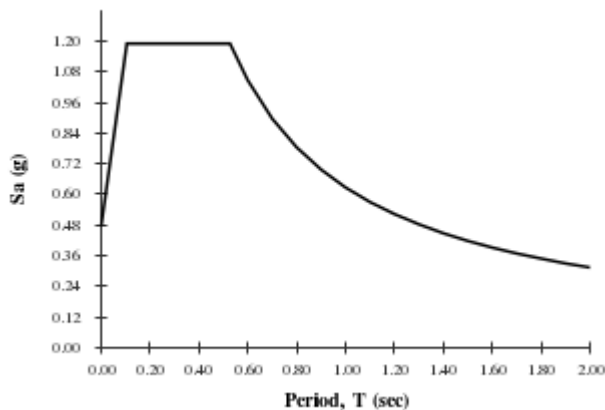


## USGS-Provided Output

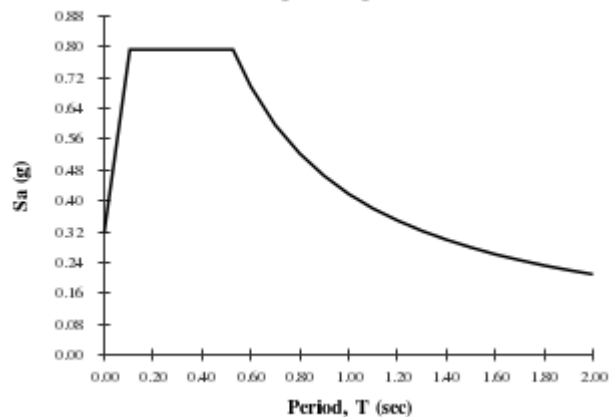
$S_s = 1.139 \text{ g}$	$S_{MS} = 1.190 \text{ g}$	$S_{DS} = 0.793 \text{ g}$
$S_1 = 0.385 \text{ g}$	$S_{M1} = 0.627 \text{ g}$	$S_{D1} = 0.418 \text{ g}$

For information on how the  $S_s$  and  $S_1$  values above have been calculated from probabilistic (risk-targeted) and deterministic ground motions in the direction of maximum horizontal response, please return to the application and select the “2009 NEHRP” building code reference document.

MCE<sub>R</sub> Response Spectrum



Design Response Spectrum



Although this information is a product of the U.S. Geological Survey, we provide no warranty, expressed or implied, as to the accuracy of the data contained therein. This tool is not a substitute for technical subject-matter knowledge.

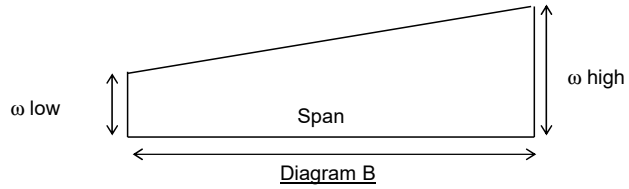
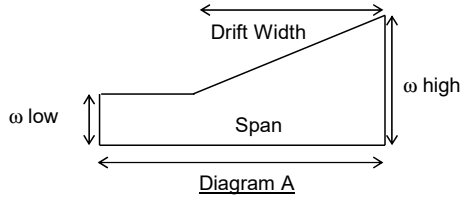


PROJECT	PROJ NO.	
	CALC NO.	
SUBJECT	Low Roof Snow Load per ASCE 7 <b>Leeward and Windward Snow Drift</b>	BY JMG DATE 1/7/20
	CHK	DATE
	SHEET	OF REV

**Description:**

**GENERAL INPUT AND OUTPUT:**

"Leeward" or "Windward":	1.00	Leeward	Snow Load Density:	19.83 pcf
Roof Slope:	8:12	33.69 °	h <sub>b</sub> (Snow Depth):	1.74 feet
County (State of Utah)	UTAH		h <sub>d</sub> (potential drift height):	-1.50 feet
Elevation at Site	4700 ft		h <sub>c</sub> (Roof to Snow):	0.00 feet
p <sub>g</sub> (SEAU Flat Roof Snow)	45 psf	----- <---Manual Entry	Maximum Drift Height:	0.00 feet
C <sub>e</sub> (Exposure Factor):	1.0		Drift Width:	0.00 feet
C <sub>t</sub> (Thermal Factor):	1.1		Maximum Snow Load:	<b>34.50 psf</b>
I <sub>s</sub> (Importance Factor):	1.0		Maximum Drift Weight:	<b>0.00 psf</b>
L <sub>u</sub> (Length of Roof):	0.00 feet	Upper Roof	Drift Width:	<b>0.00 feet</b>
Elevation Difference:	0.00 feet			
P <sub>f</sub> (Roof Snow):	<b>34.5 psf</b>			
C <sub>s</sub> (Roof Slope Factor):	1.0			
P <sub>s</sub> (Sloped Roof Snow):	<b>34.5 psf</b>			



	<b>Reeve &amp; Associates, Inc.</b> Land Planners · Civil Engineers · Land Surveyors Traffic Engineers · Structural Engineers · Landscape Architects 920 Chambers Street, Suite 14 -- Ogden, UT 84403 Phone: (801) 621-3100 Fax: (801) 621-2666			
	Date:	Sheet	Of	
	<b>1/7/2020</b>	1	1	
Designed By:	Project Number:			
<b>JMG</b>				

## Seismic Calculations

### Earthquake Loads-Site Ground Motion

$I=$	<b>1</b>	$h_n=$	<b>28.00 ft</b> (Building Height)
$R=$	6.5 Wood Brg Wall	Structure Type =	<b>Other</b>
$S_s=$	1.139	$C_t=$	0.02
$S_1=$	0.385	$x=$	0.75
Site Class=	<b>D</b>		
		Check Height for LFRS =	28.0ft > NP (per exception 12.2.5.6 where
		DL =	6.5 psf (Estimated) DL <20psf allowed up to 65ft)
$F_a=$	1.04	$S_{MS}=$	$F_a * S_s$
$S_{MS}=$	1.190	$S_{DS}=$	$2 * S_{MS} / 3$
$S_{DS}=$	0.793		
$F_v=$	1.63	$S_{M1}=$	$F_v * S_1$
$S_{M1}=$	0.628	$S_{D1}=$	$2 * S_{M1} / 3$
$S_{D1}=$	0.418		

### Earthquake Loads-Minimum Design Lateral Force

$C_s=$	0.122	$T=$	0.243 s
$C_s=$	0.264		Load Combinations that control lateral:
$C_s=$	0.035		D+0.75L+0.75S+0.75(0.7*E)
		ASD Load Factor =	<b>0.7</b>
		Rho =	<b>1.3</b>
<b>USE</b>			0.6D+0.7E
$C_s=$	<b>0.122</b>		
$V=C_s * W$		<b>V = ASD Load Factor * Rho * Cs * W =</b>	<b>0.11 * W</b>

### Dead Load Effect

$$0.2 * S_{DS} = 0.159$$

$$(+/-) 0.2 * S_{DS} * D$$

### Seismic Design Category

SDS => D

SD1 => D

Wind Design - ASCE 7-16 Chp 27

Risk Category = II  
 Basic Wind Speed V = 115 mph  
 Exposure Category = C 3  
 Wind Directionality Factor, K<sub>d</sub> = 0.85  
 Topographic Factor, K<sub>z</sub> = 1  
 Gust Effect Factor, G = 0.85  
 Total Stories = 2 (5 max)  
 Internal Pressure Coefficient, G<sub>cp</sub> = 0.18  
 -0.18

Enclosure Classification  
 Length Height Ag Ao % open Open Partial 1 Partial 2 Partial Total  
 Wall 1 = 100 10 1000 800 80.0 Y Y N N  
 Wall 2 = 100 10 1000 200 20.0 N N Y N  
 Wall 3 = 25 10 250 30 12.0 N N Y N  
 Wall 4 = 25 10 250 50 20.0 N N Y N  
 Open Building: NO  
 Partially Open: NO  
 Enclosed: YES

Wall Pressure  
 Front to Back Side to Side  
 Windward Wall, C<sub>p</sub> = 0.8 0.8  
 Windward Wall Width, B = 32 ft 49 ft  
 Side Wall Width, L = 49 ft 32 ft  
 L/B = 1.53125 0.653061  
 Leeward Wall, C<sub>p</sub> = -0.3 -0.5  
 Side Wall, C<sub>p</sub> = -0.7 -0.7  
 Roof Pressure  
 Roof Type = Gable 1 Gable 1  
 Roof Pitch = 8/12 33.7 ° 8/12 33.7 °  
 Ridge Height = 28 ft 28 ft  
 Eave Height = 18 ft 18 ft  
 Mean Roof Height, h = 22.8 ft 22.8 ft  
 h/L = 0.46 0.71  
 h/2 = 11 ft 11 ft  
 Kh = 0.93 0.93  
 qh = 26.7 psf 26.7 psf

Parapet Wall Pressure  
 Parapet Wall Height = 0 ft  
 Building Height to top of parapet = 0 ft  
 K<sub>z</sub> = 0.85  
 q<sub>p</sub> = 24.4 psf  
 WW GC<sub>p</sub> = 1.50  
 LW GC<sub>p</sub> = -1.00  
 P<sub>p</sub> = 61.1 psf  
 Adj. P<sub>p</sub> = 36.6 psf  
 Parapet Load per foot = 0.0 plf

Roof Pressure Coefficient, C <sub>p</sub>	WW Area: 548 ft <sup>2</sup> LW Area: 548 ft <sup>2</sup>				WW Area: 839 ft <sup>2</sup> LW Area: 839 ft <sup>2</sup>					
	Front to Back				Side to Side					
	Distance from Windward Edge, ft				Distance from Windward Edge, ft					
	0 ft	11 ft	23 ft	46 ft	0 ft	11 ft	23 ft	46 ft		
Windward Normal to Ridge =	Max	0.29	0.29	0.29	0.29	Max	0.24	0.24	0.24	0.24
	Min	-0.18	-0.18	-0.18	-0.18	Min	-0.21	-0.21	-0.21	-0.21
Leeward Normal to Ridge =	Max	-0.60	-0.60	-0.60	-0.60	Max	-0.60	-0.60	-0.60	-0.60
	Min	-0.60	-0.60	-0.60	-0.60	Min	-0.60	-0.60	-0.60	-0.60
Parallel to Ridge =	Max	-0.18	-0.18	-0.18	-0.18	Max	-0.18	-0.18	-0.18	-0.18
	Min	-0.90	-0.90	-0.50	-0.30	Min	-1.07	-1.07	-0.58	-0.47

Load Description	Overall Height	Wall Trib	K <sub>z</sub>	q <sub>z</sub>	Front to Back				Total	Shear Force, lbs	Adj. Shear Force, lbs	ASD Factor	
					WW	LW	SW	Int +/-					
Roof	22.8 ft	-	-	max-->	6.5 psf	-13.6 psf	varies	4.8 psf	20.1 psf	6120 lbs	3672 lbs	0.6	
				min-->	-4.1 psf	-13.6 psf	varies	-4.8 psf	9.5 psf	2902 lbs	1741 lbs		
Wall 2	8 ft	4.5	0.85		24.4 psf	16.6 psf	-6.8 psf	-15.9 psf	4.8 psf	23.4 psf	3371 lbs	2023 lbs	
Wall 1	9 ft	8.5	0.85		24.4 psf	16.6 psf	-6.8 psf	-15.9 psf	4.8 psf	23.4 psf	6368 lbs	3821 lbs	
			0.85		24.4 psf	16.6 psf	-6.8 psf	-15.9 psf	4.8 psf	23.4 psf	0 lbs	0 lbs	
			0.85		24.4 psf	16.6 psf	-6.8 psf	-15.9 psf	4.8 psf	23.4 psf	0 lbs	0 lbs	
Other			0.85		24.4 psf	16.6 psf	-6.8 psf	-15.9 psf	4.8 psf	23.4 psf	0 lbs	0 lbs	

Front to Back Total Shear: 15859 lbs 9515 lbs

Load Description	Overall Height	Wall Trib	K <sub>z</sub>	q <sub>z</sub>	Side to Side				Total	Shear Force, lbs	Adj. Shear Force, lbs	Adj. Wall Force
					WW	LW	SW	Int +/-				
Roof				max-->	5.5 psf	-13.6 psf	varies	4.8 psf	19.1 psf	8892 lbs	5335 lbs	
				min-->	-4.8 psf	-13.6 psf	varies	-4.8 psf	8.8 psf	4104 lbs	2462 lbs	
Wall 2					16.6 psf	-11.3 psf	-15.9 psf	4.8 psf	27.9 psf	6162 lbs	3697 lbs	
Wall 1					16.6 psf	-11.3 psf	-15.9 psf	4.8 psf	27.9 psf	11639 lbs	6984 lbs	
					16.6 psf	-11.3 psf	-15.9 psf	4.8 psf	27.9 psf	0 lbs	0 lbs	
					16.6 psf	-11.3 psf	-15.9 psf	4.8 psf	27.9 psf	0 lbs	0 lbs	
Other					16.6 psf	-11.3 psf	-15.9 psf	4.8 psf	27.9 psf	0 lbs	0 lbs	

Side to Side Total Shear: 26693 lbs 16016 lbs

**LATERAL ANALYSIS**

Side-to-Side Dim:	32 ft	Height	Roof Area =	1568 ft <sup>2</sup>	
Front-to-Back Dim:	49 ft		Floor Area =	1568	
Roof Trib:	4.5 ft		18 ft	Floor Area =	
Floor Trib:	8.5 ft		9 ft	Floor Area =	
Floor Trib:					
Floor Trib:					
Roof Seismic DL:	15 psf				
Floor Seismic DL:	15 psf				
Wall Seismic DL:	12 psf				

**Seismic:**                      V =    0.11    \*W

F-front-to-back:	<u>V</u>		<u>W</u>	<u>WxHx</u>	<u>Cvx</u>	<u>Fx</u>
Roof	2995 lbs	lb	26976	485568	0.64	4066 lbs
Floor	3336 lbs	lb	30048	270432	0.36	2265 lbs
Floor	0 lbs	lb	0	0	0.00	0 lbs
Floor	0 lbs	lb	0	0	0.00	0 lbs
	6331 lbs		57024	756000		6331 lbs
<b>F-side-to-side:</b>						
Roof	3199 lbs	lb	28812	518616	0.63	4375 lbs
Floor	3721 lbs	lb	33516	301644	0.37	2545 lbs
Floor	0 lbs	lb	0	0	0.00	0 lbs
Floor	0 lbs	lb	0	0	0.00	0 lbs
	6920 lbs		62328	820260		6920 lbs

**Wind:**

<b>F-front-to-back:</b>	
Roof	5695 lbs
Floor	3821 lbs
Floor	0 lbs
Floor	0 lbs
<b>F-side-to-side:</b>	
Roof	9032 lbs
Floor	6984 lbs
Floor	0 lbs
Floor	0 lbs

**Use for Design:**

<b>F-front-to-back:</b>			
Roof	5695 lbs	<b>Wind Governs</b>	1.4
Floor	3821 lbs	<b>Wind Governs</b>	1.4
Floor	0 lbs		1
Floor	0 lbs		1
<b>F-side-to-side:</b>			
Roof	9032 lbs	<b>Wind Governs</b>	1.4
Floor	6984 lbs	<b>Wind Governs</b>	1.4
Floor	0 lbs		1
Floor	0 lbs		1

	<b>SW capacities (plf):</b>		<b>Hold Down capacities (lb):</b>					
	seismic	wind	0	NONE	0	NONE		
SW-1	260	365	200	LSTD8	1610	200	MST37	1725
SW-2	380	532	1610	STHD10	2175	1725	MST48	3215
SW-3	490	685	2175	STHD14	5345	3215	MST60	5240
SW-4	640	896	5345	HDU4	4565	5240	MST72	6730
SW-5	760	1065	4565	HDU5	5645	6730	(2) MST60	10480
SW-6	980	1370	5645	HDU8	7870	10480	(2) MST72	13460
			7870	HDU11	9535	13460	NG	















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 5160 South 1500 West -- Riverdale, Utah 84405  
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 Fax: (801) 621-2666

## Spring Run 4-plex

Lehi, Utah

Date:  
**1/7/2020**

Sheet  
 1 Of  
 1

Designed By:  
**JMG**

Project Number:  
**4899-AXX**

### Force Transfer Around Openings - Diekmann Technique

Shear Line: Upper level Grid H

Horizontal Shear, V = 1311 lbs  
 Total Wall Length = 9.5 ft  
 Number of Openings = 1  
 Wall Height = 8 ft  
 Total Pier Length = 4.5 ft  
 Controlling Lateral Force = Wind

Holddown Force, H = 1104 lbs  
 D,E Shear = 220.8 lb/ft  
 B,G Shear = 291.3 lb/ft  
 Total Tension, F = 1104 lbs  
 Min Strap = 552 lbs  
 Max Strap = 552 lbs  
 Check 1 = 245.3 lb/ft  
 Check 2 = 245.3 lb/ft  
 A,C,F,H Shear = 46.0 lb/ft

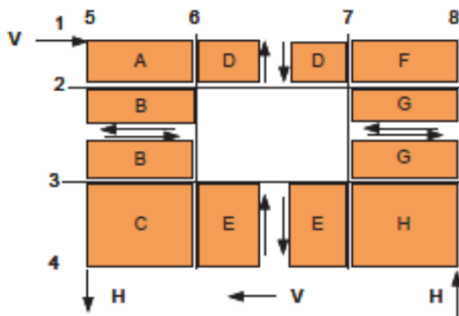
### Seismic

#	Pier		Opening		Top	Bot	Min	Red.	D-E	B-G	Max
	Left	Right	W	H	Dist	Dist	H/W	%	Sum	Sum	Strap
1	2.25	2.25	5	3	1	4	1.3	100	5	4.5	552

2.25 2.25 4.5

### Results Summary

Level	Ht	V	Vt	SW	Adj.	SW'
-	ft	lbs	lbs	-	%	-
5	8	292	292	1	100	1
4			292			
3			292			
2			292			
1			292			



### Shear Wall Capacities

	Seismic	Wind
SW-1	260 plf	365 plf
SW-2	380 plf	532 plf
SW-3	490 plf	685 plf
SW-4	640 plf	896 plf
SW-5	760 plf	1065 plf
SW-6	980 plf	1370 plf



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Shear Line: Upper level Grid F

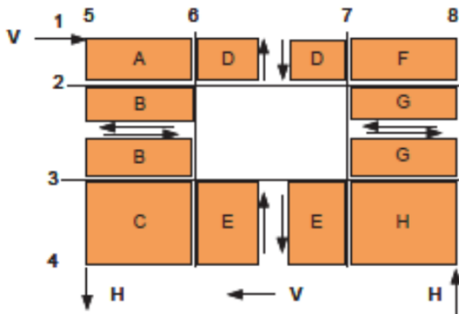
Horizontal Shear, V = 2744 lbs  
 Total Wall Length = 11 ft  
 Number of Openings = 1  
 Wall Height = 8 ft  
 Total Pier Length = 5.0 ft  
 Controlling Lateral Force = Wind

Holddown Force, H = 1996 lbs  
 D,E Shear = 665.2 lb/ft  
 B,G Shear = 548.8 lb/ft  
 Total Tension, F = 3991 lbs  
 Min Strap = **1996** lbs  
 Max Strap = **1996** lbs  
 Check 1 798.3 lb/ft  
 Check 2 798.3 lb/ft  
 A,C,F,H Shear = -249.5 lb/ft

												Seismic				
												Min	Red.	D-E	B-G	Max
												H/W	%	Sum	Sum	Strap
#	Pier Left	Pier Right	Opening		Top Dist	Bot Dist	H/W	%	D-E Sum	B-G Sum	Max Strap					
1	2.5	2.5	6	5	1	2	2.0	100	3	5	1996					
														5		

### Results Summary

Level	Ht	V	Vt	SW	Adj.	SW'
-	ft	lbs	lbs	-	%	-
5	8	665	665	3	100	3
4			665			
3			665			
2			665			
1			665			



### Shear Wall Capacities

	Seismic	Wind
SW-1	- 260 plf	365 plf
SW-2	- 380 plf	532 plf
SW-3	- 490 plf	685 plf
SW-4	- 640 plf	896 plf
SW-5	- 760 plf	1065 plf
SW-6	- 980 plf	1370 plf



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## Force Transfer Around Openings - Diekmann Technique

Shear Line: Grid T upper level - ASPEN

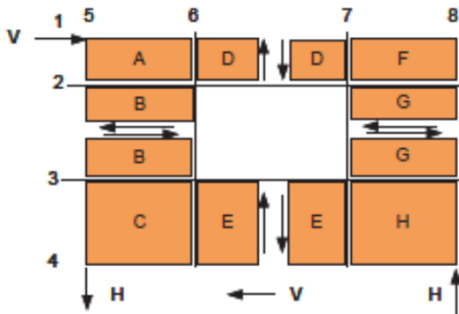
Horizontal Shear, V = 4055 lbs  
 Total Wall Length = 16 ft  
 Number of Openings = 1  
 Wall Height = 8 ft  
 Total Pier Length = 10.0 ft  
 Controlling Lateral Force = Wind

Holddown Force, H = 2028 lbs  
 D,E Shear = 675.8 lb/ft  
 B,G Shear = 405.5 lb/ft  
 Total Tension, F = 4055 lbs  
 Min Strap = 2028 lbs  
 Max Strap = 2028 lbs  
 Check 1 405.5 lb/ft  
 Check 2 405.5 lb/ft  
 A,C,F,H Shear = 0.0 lb/ft

		Seismic											
		Pier		Opening		Top	Bot	Min	Red.	D-E	B-G	Max	
#		Left	Right	W	H	Dist	Dist	H/W	%	Sum	Sum	Strap	
1	1	5	5	6	5	1	2	1.0	100	3	10	2028	
		5	5									10	

### Results Summary

Level	Ht	V	Vt	SW	Adj.	SW'
-	ft	lbs	lbs	-	%	-
5	8	676	676	3	100	3
4			676			
3			676			
2			676			
1			676			



### Shear Wall Capacities

	Seismic	Wind
SW-1	260 plf	365 plf
SW-2	380 plf	532 plf
SW-3	490 plf	685 plf
SW-4	640 plf	896 plf
SW-5	760 plf	1065 plf
SW-6	980 plf	1370 plf



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## Force Transfer Around Openings - Diekmann Technique

Shear Line: Grid T main level - ASPEN

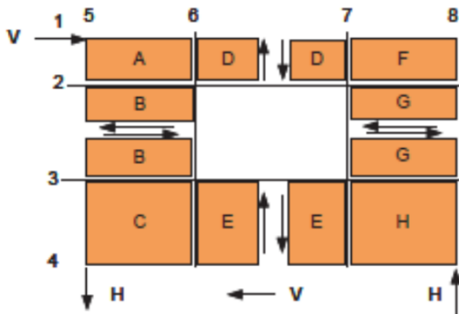
Horizontal Shear, V = 6050 lbs  
 Total Wall Length = 16 ft  
 Number of Openings = 1  
 Wall Height = 9 ft  
 Total Pier Length = 10.0 ft  
 Controlling Lateral Force = Wind

Holdown Force, H = 3403 lbs  
 D,E Shear = 850.8 lb/ft  
 B,G Shear = 605 lb/ft  
 Total Tension, F = 5105 lbs  
 Min Strap = 2552 lbs  
 Max Strap = 2552 lbs  
 Check 1 510.5 lb/ft  
 Check 2 510.5 lb/ft  
 A,C,F,H Shear = 94.5 lb/ft

												Seismic					
												Min	Red.	D-E	B-G	Max	
												H/W	%	Sum	Sum	Strap	
#	Pier Left	Pier Right	Opening W H		Top Dist	Bot Dist											
1	5	5	6	5	2	2	1.0	100	4	10	2552						
														10			

### Results Summary

Level	Ht	V	Vt	SW	Adj.	SW'
-	ft	lbs	lbs	-	%	-
5	8	851	851	4	100	4
4			851			
3			851			
2			851			
1			851			



### Shear Wall Capacities

	Seismic	Wind
SW-1	260 plf	365 plf
SW-2	380 plf	532 plf
SW-3	490 plf	685 plf
SW-4	640 plf	896 plf
SW-5	760 plf	1065 plf
SW-6	980 plf	1370 plf



Simple Span Beam Calculation

Adjustment Factors - ASD

Cd:	1.15	Cfu:	NA
Cm:	1.00	Ci:	1.00
Ct:	1.00	Cr:	1.00
Ci:	1.00	Cv:	-
Cf:	-	Cc:	1.00

Glu-Lam & LVL Only  
Glu-Lam Only

Roof DL	10	psf	Include Self Wt?
Floor DL	15	psf	No
Live	40	psf	
Snow	35	psf	

---> Point Load distance must be >= midspan distance!

Active Member for RB-6  
Deflection Calc and Shear Diagram:

member ID	span ft	roof trib ft	floor trib ft	wall load plf	point load			dist from left, ft	left reaction				right reaction				applied moment lb-ft	% Str	applied shear lbs	% Str	Selected Member	Live/Total Deflection	actual Δ, in.	Δ limit, L/	Δ act, L/	check	Max Defl Location, ft	Min Bearing Length, in
					dead lbs	live lbs	snow lbs		dead lbs	live lbs	snow lbs	dead lbs	live lbs	snow lbs	dead lbs	live lbs												
RB-1	4	21						420	0	1470	420	0	1470	1890	69.5	1890	63.0	(2) 2x8	Live	0.03	L/240	L/1728	Pass	2.0	1.0			
RB-2	6	5						150	0	525	150	0	525	1013	37.2	675	22.5	(2) 2x8	Live	0.03	L/240	L/2151	Pass	3.0	0.4			
RB-3	6	26						780	0	2730	780	0	2730	5265	64.4	3510	63.3	(2) 1-3/4"x7-1/4" LVL	Live	0.12	L/240	L/603	Pass	3.0	1.3			
RB-4	8	8						320	0	1120	320	0	1120	2880	70.6	1440	32.0	(3) 2x8	Live	0.11	L/240	L/851	Pass	4.0	0.5			
RB-5	10.5	19			700		1650	8.5	1131	0	3806	1564	0	4827	14250	73.7	6391	60.2	(3) 1-3/4"x9-1/4" LVL	Live	0.32	L/360	L/393	Pass	5.4	1.6		
RB-6	14	5		100				1050	0	1225	1050	0	1225	7963	41.2	2275	21.4	(3) 1-3/4"x9-1/4" LVL	Live	0.22	L/360	L/769	Pass	7.0	0.6			
RB-7								0	0	0	0	0	0	0		0			Live		L/360							
RB-8								0	0	0	0	0	0	0		0			Live		L/360							
RB-9								0	0	0	0	0	0	0		0			Live		L/360							
RB-10								0	0	0	0	0	0	0		0			Live		L/360							
RB-11								0	0	0	0	0	0	0		0			Live		L/360							
RB-12								0	0	0	0	0	0	0		0			Live		L/360							
RB-13								0	0	0	0	0	0	0		0			Live		L/360							
RB-14								0	0	0	0	0	0	0		0			Live		L/360							
RB-15								0	0	0	0	0	0	0		0			Live		L/360							
RB-16								0	0	0	0	0	0	0		0			Live		L/360							
RB-17								0	0	0	0	0	0	0		0			Live		L/360							
RB-18								0	0	0	0	0	0	0		0			Live		L/360							
RB-19								0	0	0	0	0	0	0		0			Live		L/360							
RB-20								0	0	0	0	0	0	0		0			Live		L/360							
RB-21								0	0	0	0	0	0	0		0			Live		L/360							
RB-22								0	0	0	0	0	0	0		0			Live		L/360							
RB-23								0	0	0	0	0	0	0		0			Live		L/360							
RB-24								0	0	0	0	0	0	0		0			Live		L/360							
RB-25								0	0	0	0	0	0	0		0			Live		L/360							
RB-26								0	0	0	0	0	0	0		0			Live		L/360							
RB-27								0	0	0	0	0	0	0		0			Live		L/360							

Simple Span Beam Calculation

Adjustment Factors - ASD

Cd:	1.00	Cfu:	NA
Cm:	1.00	Ci:	1.00
Ct:	1.00	Cr:	1.15
Cl:	1.00	Cv:	-
Cf:	-	Cc:	1.00

Glu-Lam & LVL Only  
Glu-Lam Only

Roof DL	15	psf	Include Self Wt?
Floor DL	15	psf	No
Live	40	psf	
Snow	35	psf	

---> Point Load distance must be >= midspan distance!

Active Member for Deflection Calc and Shear Diagram: FB-21

member ID	span ft	roof trib ft	floor trib ft	wall load plf	point load			dist from left, ft	left reaction				right reaction				applied moment lb-ft	% Str	applied shear lbs	% Str	Selected Member	Live/Total Deflection	actual Δ, in.	Δ limit L/	Δ act L/	check	Max Defl Location, ft	Min Bearing Length, in
					dead lbs	live lbs	snow lbs		dead lbs	live lbs	snow lbs	dead lbs	live lbs	snow lbs														
FB-1	4		16					480	1280	0	480	1280	0	1760	64.7	1760	67.4	(2) 2x8	Live	0.04	L/360	L/1227	Pass	3.2	0.9			
FB-2	6		13					585	1560	0	585	1560	0	3218	78.9	2145	54.8	(3) 2x8	Live	0.07	L/360	L/1086	Pass	3.0	0.8			
FB-3	6		13					585	1560	0	585	1560	0	3218	79.3	2145	64.4	(2) 2x10	Live	0.05	L/360	L/1503	Pass	3.0	1.1			
FB-4	4		7					210	560	0	210	560	0	770	3.8	770	9.8	(2) 1-3/4"x11-7/8" LVL	Live	0.00	L/480	L/29072	Pass	2.0	0.3			
FB-5	3	22	6					630	360	1155	630	360	1155	1339	49.2	1785	68.4	(2) 2x8	Live	0.01	L/480	L/3910	Pass	1.5	1.1			
FB-6	10	5	8	100				1475	1600	875	1475	1600	875	8328	40.6	3331	42.2	(2) 1-3/4"x11-7/8" LVL	Live	0.09	L/480	L/1403	Pass	5.0	1.5			
FB-7	5.5	18	5	100				1224	550	1733	1224	550	1733	4065	19.8	2956	37.4	(2) 1-3/4"x11-7/8" LVL	Live	0.01	L/480	L/4970	Pass	2.7	1.3			
FB-8	4.5	8	7	100				731	630	630	731	630	630	1886	9.2	1676	21.2	(2) 1-3/4"x11-7/8" LVL	Live	0.00	L/480	L/13612	Pass	2.2	0.8			
FB-9	20							0	0	0	0	0	0	0	#N/A	0	#N/A	see calc	Live		L/360	#####	#####					
FB-10	20							0	0	0	0	0	0	0	#N/A	0	#N/A	see calc	Live		L/480	#####	#####					
FB-11	16							0	0	0	0	0	0	0	#N/A	0	#N/A	see calc	Live		L/480	#####	#####					
FB-12								0	0	0	0	0	0	0				not used	Live		L/480				6.5			
FB-13	15		14					1575	4200	0	1575	4200	0	21656	70.3	5775	48.8	(3) 1-3/4"x11-7/8" LVL	Live	0.44	L/360	L/413	Pass	7.5	1.5			
FB-14	3		28	100				780	1680	0	780	1680	0	1845	45.5	2460	73.9	(2) 2x10	Live	0.01	L/480	L/5583	Pass	1.5	1.3			
FB-15	8		18	100				1480	2880	0	1480	2880	0	8720	71.1	4360	60.3	(3) 1-3/4"x7-1/4" LVL	Live	0.20	L/480	L/482	Pass	4.0	1.1			
FB-16								0	0	0	0	0	0	0					Live		L/480							
FB-17	5		22					825	2200	0	825	2200	0	3781	51.6	3025	55.1	(3) 1-3/4"x5-1/2" LVL	Live	0.09	L/480	L/706	Pass	2.5	0.8			
FB-18								0	0	0	0	0	0	0					Live		L/480							
FB-19	20.5		8					1230	3280	0	1230	3280	0	23114	53.5	4510	33.2	5-1/8"x15" GLB	Live	0.49	L/480	L/502	Pass	10.2	1.4			
FB-20	8		5					300	800	0	300	800	0	2200	54.2	1100	33.0	(2) 2x10	Live	0.06	L/480	L/1649	Pass	4.0	0.6			
FB-21	14		3					315	840	0	315	840	0	4043	30.6	1155	19.9	3-1/8"x10-1/2" GLB	Live	0.19	L/480	L/879	Pass	7.0	0.6			
FB-22								0	0	0	0	0	0	0					Live		L/480							
FB-23								0	0	0	0	0	0	0					Live		L/480							
FB-24								0	0	0	0	0	0	0					Live		L/480							
FB-25								0	0	0	0	0	0	0					Live		L/480							
FB-26								0	0	0	0	0	0	0					Live		L/480							
FB-27								0	0	0	0	0	0	0					Live		L/480							

FOOTINGS:

ALLOWABLE BEARING PRESSURE = 1.5 KSF  
 Assumed Coefficient of Friction Soil to Foundation = 0.3

Frame Line	GRAVITY									Factored			Required	Specified
	DL	LL	SL	Trib <sub>DL</sub>	Trib <sub>LL</sub>	Trib <sub>SL</sub>	Wall	Trib <sub>Wall</sub>	DL	LL	SL	FTG	FTG	
Grid L	L	15.0 psf	40.0 psf	35.0 psf	28.0ft	28.0ft	0.0ft	53.0 psf	17.0ft	1.3 k	1.1k	19.5 in	FC- 24	
rear left	T	15.0 psf	40.0 psf	35.0 psf	28.0ft	7.0ft	21.0ft	45.0 psf	21.0ft	1.4 k	0.8k	17.0 in	FC- 20	
rear right	T	15.0 psf	40.0 psf	35.0 psf	19.0ft	14.0ft	5.0ft	40.0 psf	25.0ft	1.3 k	0.6k	14.8 in	FC- 20	
side	1	15.0 psf	40.0 psf	35.0 psf	10.0ft	5.0ft	5.0ft	40.0 psf	25.0ft	1.2 k	0.3k	11.5 in	FC- 20	
marriage	2	15.0 psf	40.0 psf	35.0 psf	21.0ft	4.0ft	17.0ft	15.0 psf	50.0ft	1.1 k	0.6k	13.3 in	FC- 20	
Grid P	P	15.0 psf	40.0 psf	35.0 psf	28.0ft	28.0ft	0.0ft	12.0 psf	17.0ft	0.6 k	1.1k	14.0 in	FC- 18	
Grid 1.4	1.4	15.0 psf	40.0 psf	35.0 psf	14.0ft	5.0ft	9.0ft	12.0 psf	17.0ft	0.4 k	0.4k	6.4 in	FC- 18	
SPOT FTGS														
	B	0.0 psf	0.0 psf	0.0 psf	0.0sf	0.0sf	0.0sf	0.0 psf	0.0sf	0.0 k	0.0k	0.0 in	FS- 0	
F porch	C	15.0 psf	40.0 psf	35.0 psf	50.0sf	0.0sf	25.0sf	0.0 psf	0.0sf	0.8 k	0.9k	12.5 in	FS- 24	
1.4 - P	D	15.0 psf	40.0 psf	35.0 psf	205.0sf	105.0sf	100.0sf	0.0 psf	0.0sf	3.1 k	5.8k	29.1 in	FS- 36	

TYPICAL FOOTINGS AND REINFORCING

Footing Parameters			
Soil Bearing Pressure = 1500 psf	f <sub>c</sub> = 2500 psi	Note: Not all footings were used on this project	
	f <sub>y</sub> steel = 60 ksi		
	p <sub>max</sub> = 0.016		

Continuous Footings						Trial Reinf				
Callout	Typ Eccent	Allowable Load	Ftg Req'd	Nom Ftg	Asmin/ft	As Req'd/ft	Num Bar	Bar #	As Prov	/ As
FC-1.5	0.1%	<u>2.2 klf</u>	1.48'	<u>1.5'</u>	0.18 in <sup>2</sup>	0.36 in <sup>2</sup>	(2)	4	0.4	OK
FC-2.0	0.1%	<u>3.0 klf</u>	1.98'	<u>2.0'</u>	0.24 in <sup>2</sup>	0.48 in <sup>2</sup>	(3)	4	0.6	OK
FC-2.5	0.1%	<u>3.7 klf</u>	2.48'	<u>2.5'</u>	0.30 in <sup>2</sup>	0.60 in <sup>2</sup>	(3)	5	0.93	OK
FC-3.0	0.1%	<u>4.5 klf</u>	2.98'	<u>3.0'</u>	0.36 in <sup>2</sup>	0.72 in <sup>2</sup>	(3)	5	0.93	OK
FC-3.5	0.1%	<u>5.2 klf</u>	3.48'	<u>3.5'</u>	0.42 in <sup>2</sup>	0.84 in <sup>2</sup>	(3)	5	0.93	OK
FC-4.0	0.1%	<u>6.0 klf</u>	3.98'	<u>4.0'</u>	0.48 in <sup>2</sup>	0.96 in <sup>2</sup>	(4)	5	1.24	OK
FC-4.5	0.1%	<u>6.7 klf</u>	4.48'	<u>4.5'</u>	0.54 in <sup>2</sup>	1.08 in <sup>2</sup>	(4)	5	1.24	OK
FC-5.0	0.1%	<u>7.5 klf</u>	4.98'	<u>5.0'</u>	0.60 in <sup>2</sup>	1.20 in <sup>2</sup>	(5)	5	1.55	OK
FTS-1.5	0.1%	<u>2.2 klf</u>	1.48'	<u>1.5'</u>	0.18 in <sup>2</sup>	0.36 in <sup>2</sup>	(2)	4	0.4	OK
FTS-2.0	0.1%	<u>3.0 klf</u>	1.98'	<u>2.0'</u>	0.24 in <sup>2</sup>	0.48 in <sup>2</sup>	(3)	4	0.6	OK
FTS-2.5	0.1%	<u>3.7 klf</u>	2.48'	<u>2.5'</u>	0.30 in <sup>2</sup>	0.60 in <sup>2</sup>	(3)	5	0.93	OK
FTS-3.0	0.1%	<u>4.5 klf</u>	2.98'	<u>3.0'</u>	0.36 in <sup>2</sup>	0.72 in <sup>2</sup>	(3)	5	0.93	OK
FTS-3.5	0.1%	<u>5.2 klf</u>	3.48'	<u>3.5'</u>	0.42 in <sup>2</sup>	0.84 in <sup>2</sup>	(3)	5	0.93	OK

Spot Footings							Trial Reinf						
Callout	Typ Eccent	Allowable Load	Ftg Req'd	Nom Ftg	Mu	p	/ p	Asmin	As Req'd	Num Bar	Bar #	As Prov	/ As
FS-2.0	0.1%	<u>5.9 k</u>	1.99'	<u>2.0'</u>	1.02 k-ft	0.000	OK	0.43 in <sup>2</sup>	0.05 in <sup>2</sup>	(2)	5	0.62	OK
FS-2.5	0.1%	<u>9.2 k</u>	2.49'	<u>2.5'</u>	2.16 k-ft	0.000	OK	0.54 in <sup>2</sup>	0.10 in <sup>2</sup>	(3)	5	0.93	OK
FS-3.0	0.1%	<u>13.3 k</u>	2.99'	<u>3.0'</u>	3.95 k-ft	0.001	OK	0.65 in <sup>2</sup>	0.18 in <sup>2</sup>	(3)	5	0.93	OK
FS-3.5	0.1%	<u>18.2 k</u>	3.49'	<u>3.5'</u>	6.52 k-ft	0.000	OK	0.90 in <sup>2</sup>	0.22 in <sup>2</sup>	(3)	5	0.93	OK
FS-4.0	0.1%	<u>23.8 k</u>	3.99'	<u>4.0'</u>	10.02 k-ft	0.001	OK	1.03 in <sup>2</sup>	0.33 in <sup>2</sup>	(4)	5	1.24	OK
FS-4.5	0.1%	<u>30.1 k</u>	4.49'	<u>4.5'</u>	14.58 k-ft	0.001	OK	1.16 in <sup>2</sup>	0.48 in <sup>2</sup>	(4)	5	1.24	OK
FS-5.0	0.1%	<u>37.2 k</u>	4.99'	<u>5.0'</u>	19.63 k-ft	0.001	OK	1.29 in <sup>2</sup>	0.65 in <sup>2</sup>	(5)	5	1.55	OK
FS-5.5	0.1%	<u>45.0 k</u>	5.50'	<u>5.5'</u>	26.59 k-ft	0.001	OK	1.42 in <sup>2</sup>	0.89 in <sup>2</sup>	(5)	5	1.55	OK
FS-6.0	0.1%	<u>53.5 k</u>	5.99'	<u>6.0'</u>	33.81 k-ft	0.001	OK	1.60 in <sup>2</sup>	1.07 in <sup>2</sup>	(6)	5	1.86	OK

PUNCHING SHEAR CHECK

Note: Not all footings were used on this project

Callout	Factored Load	Min Col. Dim	factored qu	Assumed depth	d	bo	Vu	Vc	/ Capacity
FS-2.0	8.5 k	4"	2.12 ksf	10"	7.0"	44"	6.7 k	52.4 k	OK
FS-2.5	13.3 k	4"	2.13 ksf	10"	7.0"	44"	11.5 k	52.4 k	OK
FS-3.0	19.2 k	4"	2.13 ksf	10"	7.0"	44"	17.4 k	52.4 k	OK
FS-3.5	26.2 k	4"	2.14 ksf	12"	9.0"	52"	23.7 k	79.6 k	OK
FS-4.0	34.2 k	4"	2.14 ksf	12"	9.0"	52"	31.7 k	79.6 k	OK
FS-4.5	43.4 k	4"	2.14 ksf	12"	9.0"	52"	40.8 k	79.6 k	OK
FS-5.0	53.6 k	5"	2.14 ksf	12"	9.0"	56"	50.7 k	85.7 k	OK
FS-5.5	64.9 k	5"	2.14 ksf	12"	9.0"	56"	61.9 k	85.7 k	OK
FS-6.0	77.0 k	6"	2.14 ksf	12"	9.4"	62"	73.5 k	98.4 k	OK

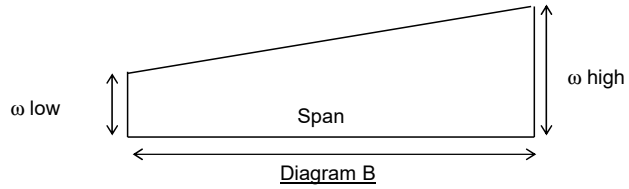
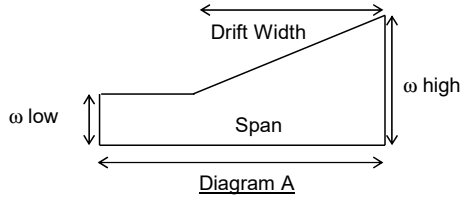


PROJECT		PROJ NO.	
		CALC NO.	
SUBJECT	<b>Low Roof Snow Load per ASCE 7 Leeward and Windward Snow Drift</b>	BY JMG	DATE 1/8/20
		CHK	DATE
		SHEET	OF REV

**Description:**

**GENERAL INPUT AND OUTPUT:**

"Leeward" or "Windward":	1.00	Leeward	Snow Load Density:	19.83 pcf
Roof Slope:	8:12	33.69 °	h <sub>b</sub> (Snow Depth):	1.74 feet
County (State of Utah)	UTAH		h <sub>d</sub> (potential drift height):	-1.50 feet
Elevation at Site	4700 ft		h <sub>c</sub> (Roof to Snow):	0.00 feet
p <sub>g</sub> (SEAU Flat Roof Snow)	45 psf	<---Manual Entry	Maximum Drift Height:	0.00 feet
C <sub>e</sub> (Exposure Factor):	1.0		Drift Width:	0.00 feet
C <sub>t</sub> (Thermal Factor):	1.1		Maximum Snow Load:	<b>34.50 psf</b>
I <sub>s</sub> (Importance Factor):	1.0		Maximum Drift Weight:	<b>0.00 psf</b>
L <sub>u</sub> (Length of Roof):	0.00 feet	Upper Roof	Drift Width:	<b>0.00 feet</b>
Elevation Difference:	0.00 feet			
P <sub>f</sub> (Roof Snow):	<b>34.5 psf</b>			
C <sub>s</sub> (Roof Slope Factor):	1.0			
P <sub>s</sub> (Sloped Roof Snow):	<b>34.5 psf</b>			



	<b>Reeve &amp; Associates, Inc.</b> Land Planners · Civil Engineers · Land Surveyors Traffic Engineers · Structural Engineers · Landscape Architects 920 Chambers Street, Suite 14 -- Ogden, UT 84403 Phone: (801) 621-3100 Fax: (801) 621-2666			
	Date:	Sheet	Of	
	<b>1/8/2020</b>	1	1	
Designed By:	Project Number:			
<b>JMG</b>				

## Seismic Calculations

### Earthquake Loads-Site Ground Motion

$I =$	1	$h_n =$	28.00 ft (Building Height)
$R =$	6.5 Wood Brg Wall	Structure Type =	Other
$S_s =$	1.139	$C_t =$	0.02
$S_1 =$	0.385	$x =$	0.75
Site Class =	D		
		Check Height for LFRS =	28.0ft > NP (per exception 12.2.5.6 where
		DL =	6.5 psf (Estimated) DL < 20psf allowed up to 65ft)
$F_a =$	1.04	$S_{MS} =$	$F_a * S_s$
$S_{MS} =$	1.190	$S_{DS} =$	$2 * S_{MS} / 3$
$S_{DS} =$	0.793		
$F_v =$	1.63	$S_{M1} =$	$F_v * S_1$
$S_{M1} =$	0.628	$S_{D1} =$	$2 * S_{M1} / 3$
$S_{D1} =$	0.418		

### Earthquake Loads-Minimum Design Lateral Force

$C_s =$	0.122	$T =$	0.243 s
$C_s =$	0.264		Load Combinations that control lateral:
$C_s =$	0.035		D+0.75L+0.75S+0.75(0.7*E)
		ASD Load Factor =	0.7
USE		Rho =	1.3
$C_s =$	0.122		0.6D+0.7E
$V = C_s * W$			
		$V = \text{ASD Load Factor} * \text{Rho} * C_s * W =$	0.11 * W

### Dead Load Effect

$$0.2 * S_{DS} = 0.159$$

$$(+/-) 0.2 * S_{DS} * D$$

### Seismic Design Category

SDS => D

SD1 => D

Wind Design - ASCE 7-16 Chp 27

Risk Category = II  
 Basic Wind Speed V = 115 mph  
 Exposure Category = C  
 Wind Directionality Factor, K<sub>d</sub> = 0.85  
 Topographic Factor, K<sub>t</sub> = 1  
 Gust Effect Factor, G = 0.85  
 Total Stories = 2 (5 max)  
 Internal Pressure Coefficient, Gcpi = 0.18  
 -0.18

Enclosure Classification				Ag	Ao	% open	Open	Partial 1	Partial 2	Partial Total
Length	Height									
Wall 1 =	100	10	1000	800	80.0	Y	Y	N	N	N
Wall 2 =	100	10	1000	200	20.0	N	N	Y	N	N
Wall 3 =	25	10	250	30	12.0	N	N	Y	N	N
Wall 4 =	25	10	250	50	20.0	N	N	Y	N	N

Open Building: NO  
 Partially Open: NO  
 Enclosed: YES

Wall Pressure	Front to Back	Side to Side
Windward Wall, C <sub>p</sub> =	0.8	0.8
Windward Wall Width, B =	30 ft	53 ft
Side Wall Width, L =	53 ft	30 ft
L/B =	1.766667	0.566038
Leeward Wall, C <sub>p</sub> =	-0.3	-0.5
Side Wall, C <sub>p</sub> =	-0.7	-0.7

Parapet Wall Pressure	
Parapet Wall Height =	0 ft
Building Height to top of parapet =	0 ft
K <sub>z</sub> =	0.85
q <sub>p</sub> =	24.4 psf
WW GC <sub>p</sub> =	1.50
LW GC <sub>p</sub> =	-1.00
P <sub>p</sub> =	61.1 psf
Adj. P <sub>p</sub> =	36.6 psf
Parapet Load per foot =	0.0 plf

Roof Pressure	Gable 1	Gable 2
Roof Type =	Gable 1	Gable 2
Roof Pitch =	8/12	8/12
Ridge Height =	28 ft	28 ft
Eave Height =	18 ft	18 ft
Mean Roof Height, h =	22.8 ft	22.8 ft
h/L =	0.43	0.76
h/2 =	11 ft	11 ft
Kh =	0.93	0.93
qh =	26.7 psf	26.7 psf

Roof Pressure Coefficient, C <sub>p</sub>	WW Area: 514 ft <sup>2</sup>   LW Area: 514 ft <sup>2</sup>				WW Area: 908 ft <sup>2</sup>   LW Area: 908 ft <sup>2</sup>				
	Front to Back				Side to Side				
	Distance from Windward Edge, ft				Distance from Windward Edge, ft				
	0 ft	11 ft	23 ft	46 ft	0 ft	11 ft	23 ft	46 ft	
Windward Normal to Ridge =	Max	0.30	0.30	0.30	0.30	Max	0.24	0.24	0.24
	Min	-0.16	-0.16	-0.16	-0.16	Min	-0.21	-0.21	-0.21
Leeward Normal to Ridge =	Max	-0.60	-0.60	-0.60	-0.60	Max	-0.60	-0.60	-0.60
	Min	-0.60	-0.60	-0.60	-0.60	Min	-0.60	-0.60	-0.60
Parallel to Ridge =	Max	-0.18	-0.18	-0.18	-0.18	Max	-0.18	-0.18	-0.18
	Min	-0.90	-0.90	-0.50	-0.30	Min	-1.11	-1.11	-0.51

Load Description	Overall Height	Wall Trib	K <sub>z</sub>	q <sub>z</sub>	Front to Back				Total	Shear Force, lbs	Adj. Shear Force, lbs	ASD Factor	
					WW	LW	SW	Int +/-					
Roof	22.8 ft	-	-	max-->	6.8 psf	-13.6 psf	varies	4.8 psf	20.4 psf	5828 lbs	3497 lbs	0.6	
				min-->	-3.6 psf	-13.6 psf	varies	-4.8 psf	10.0 psf	2854 lbs	1712 lbs		
Wall 2	8 ft	4.5	0.85		24.4 psf	16.6 psf	-6.8 psf	-15.9 psf	4.8 psf	23.4 psf	3161 lbs	1896 lbs	
Wall 1	9 ft	8.5	0.85		24.4 psf	16.6 psf	-6.8 psf	-15.9 psf	4.8 psf	23.4 psf	5970 lbs	3582 lbs	
			0.85		24.4 psf	16.6 psf	-6.8 psf	-15.9 psf	4.8 psf	23.4 psf	0 lbs	0 lbs	
			0.85		24.4 psf	16.6 psf	-6.8 psf	-15.9 psf	4.8 psf	23.4 psf	0 lbs	0 lbs	
Other			0.85		24.4 psf	16.6 psf	-6.8 psf	-15.9 psf	4.8 psf	23.4 psf	0 lbs	0 lbs	
Front to Back Total Shear:										14958 lbs	8975 lbs		

Load Description	Overall Height	Wall Trib	K <sub>z</sub>	q <sub>z</sub>	Side to Side				Total	Shear Force, lbs	Adj. Shear Force, lbs	Adj. Wall Force
					WW	LW	SW	Int +/-				
Roof				max-->	5.3 psf	-13.6 psf	varies	4.8 psf	18.9 psf	9538 lbs	5723 lbs	
				min-->	-4.8 psf	-13.6 psf	varies	-4.8 psf	8.8 psf	4411 lbs	2646 lbs	
Wall 2					16.6 psf	-11.3 psf	-15.9 psf	4.8 psf	27.9 psf	6665 lbs	3999 lbs	
Wall 1					16.6 psf	-11.3 psf	-15.9 psf	4.8 psf	27.9 psf	12589 lbs	7554 lbs	
					16.6 psf	-11.3 psf	-15.9 psf	4.8 psf	27.9 psf	0 lbs	0 lbs	
					16.6 psf	-11.3 psf	-15.9 psf	4.8 psf	27.9 psf	0 lbs	0 lbs	
Other					16.6 psf	-11.3 psf	-15.9 psf	4.8 psf	27.9 psf	0 lbs	0 lbs	
Side to Side Total Shear:										28792 lbs	17275 lbs	

**LATERAL ANALYSIS**

Side-to-Side Dim:	30 ft	Height	Roof Area =	1590 ft <sup>2</sup>
Front-to-Back Dim:	53 ft		Floor Area =	1590
Roof Trib:	4.5 ft	18 ft	Floor Area =	
Floor Trib:	8.5 ft	9 ft	Floor Area =	
Floor Trib:				
Floor Trib:				
Roof Seismic DL:	15 psf			
Floor Seismic DL:	15 psf			
Wall Seismic DL:	12 psf			

**Seismic:** V = 0.11 \*W

F-front-to-back:	<u>V</u>	<u>W</u>	<u>WxHx</u>	<u>Cvx</u>	<u>Fx</u>
Roof	3008 lbs lb	27090	487620	0.64	4079 lbs
Floor	3327 lbs lb	29970	269730	0.36	2256 lbs
Floor	0 lbs lb	0	0	0.00	0 lbs
Floor	0 lbs lb	0	0	0.00	0 lbs
	6335 lbs	57060	757350		6335 lbs

F-side-to-side:

Roof	3284 lbs lb	29574	532332	0.63	4497 lbs
Floor	3848 lbs lb	34662	311958	0.37	2635 lbs
Floor	0 lbs lb	0	0	0.00	0 lbs
Floor	0 lbs lb	0	0	0.00	0 lbs
	7132 lbs	64236	844290		7132 lbs

**Wind:**

F-front-to-back:

Roof	5393 lbs
Floor	3582 lbs
Floor	0 lbs
Floor	0 lbs

F-side-to-side:

Roof	9722 lbs
Floor	7554 lbs
Floor	0 lbs
Floor	0 lbs

**Use for Design:**

F-front-to-back:

Roof	5393 lbs	<b>Wind Governs</b>	1.4
Floor	3582 lbs	<b>Wind Governs</b>	1.4
Floor	0 lbs		1
Floor	0 lbs		1

F-side-to-side:

Roof	9722 lbs	<b>Wind Governs</b>	1.4
Floor	7554 lbs	<b>Wind Governs</b>	1.4
Floor	0 lbs		1
Floor	0 lbs		1

**SW capacities (plf):**

	seismic	wind
SW-1	260	365
SW-2	380	532
SW-3	490	685
SW-4	640	896
SW-5	760	1065
SW-6	980	1370

**Hold Down capacities (lb):**

0	NONE	0	NONE
200	LSTD8 1610	200	MST37 1725
1610	STHD10 2175	1725	MST48 3215
2175	STHD14 3500	3215	MST60 5240
3500	HDU4 4565	5240	MST72 6730
4565	HDU5 5645	6730	(2) MST60 10480
5645	HDU8 7870	10480	(2) MST72 13460
7870	HDU11 9535	13460	NG
9535	HDU14 14445		















# Reeve & Associates, Inc.

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Spring Run 4-plex

Lehi, Utah

Date: 1/7/2020 Sheet 1 Of 1

Designed By: JMG

Project Number: 4899-AXX

## Force Transfer Around Openings - Diekmann Technique

Shear Line: upper level rear wall - DOVER

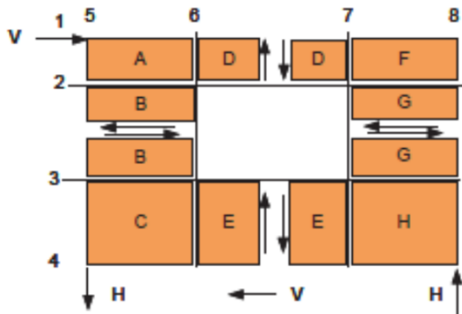
Horizontal Shear, V = 4952 lbs  
 Total Wall Length = 14 ft  
 Number of Openings = 1  
 Wall Height = 11.75 ft  
 Total Pier Length = 8.0 ft  
 Controlling Lateral Force = Wind

Holddown Force, H = 4156 lbs  
 D,E Shear = 977.9 lb/ft  
 B,G Shear = 619 lb/ft  
 Total Tension, F = 5867 lbs  
 Min Strap = 2934 lbs  
 Max Strap = 2934 lbs  
 Check 1 = 733.4 lb/ft  
 Check 2 = 733.4 lb/ft  
 A,C,F,H Shear = -114.4 lb/ft

#	Pier		Opening		Top	Bot	Seismic				
	Left	Right	W	H	Dist	Dist	Min H/W	Red. %	D-E Sum	B-G Sum	Max Strap
1	4	4	6	7.5	2.25	2	1.9	100	4.25	8	2934
	4	4									

### Results Summary

Level	Ht ft	V lbs	Vt lbs	SW	Adj. %	SW'
-						
5	8	851	851	4	100	4
4			851			
3			851			
2			851			
1			851			



### Shear Wall Capacities

	Seismic	Wind
SW-1	260 plf	365 plf
SW-2	380 plf	532 plf
SW-3	490 plf	685 plf
SW-4	640 plf	896 plf
SW-5	760 plf	1065 plf
SW-6	980 plf	1370 plf



Simple Span Beam Calculation

Adjustment Factors - ASD

Cd:	1.00	Cfu:	NA
Cm:	1.00	Ci:	1.00
Ct:	1.00	Cr:	1.15
Ci:	1.00	Cv:	-
Cf:	-	Cc:	1.00

Glu-Lam & LVL Only  
Glu-Lam Only

Roof DL	15	psf	Include Self Wt?
Floor DL	15	psf	No
Live	40	psf	
Snow	35	psf	

---> Point Load distance must be >= midspan distance!

Active Member for Deflection Calc and Shear Diagram: FB-17

member ID	span ft	roof trib ft	floor trib ft	wall load plf	point load			dist from left, ft	left reaction				right reaction				applied moment lb-ft	% Str	applied shear lbs	% Str	Selected Member	Live/Total Deflection	actual Δ, in.	Δ limit L/	Δ act L/	check	Max Defl Location, ft	Min Bearing Length, in
					dead lbs	live lbs	snow lbs		dead lbs	live lbs	snow lbs	dead lbs	live lbs	snow lbs	dead lbs	live lbs												
FB-1	4		16						480	1280	0	480	1280	0	1760	64.7	1760	67.4	(2) 2x8	Live	0.04	L/360	L/1227	Pass	3.2	0.9		
FB-2	6		13						585	1560	0	585	1560	0	3218	78.9	2145	54.8	(3) 2x8	Live	0.07	L/360	L/1086	Pass	3.0	0.8		
FB-3	6		13						585	1560	0	585	1560	0	3218	79.3	2145	64.4	(2) 2x10	Live	0.05	L/360	L/1503	Pass	3.0	1.1		
FB-4	4		7						210	560	0	210	560	0	770	7.5	770	19.5	1-3/4"x11-7/8" LVL	Live	0.00	L/480	L/14536	Pass	2.0	0.6		
FB-5	14	26	3	80					3605	840	6370	3605	840	6370	34913	83.4	9975	71.4	(3) 1-3/4"x14" LVL	Live	0.33	L/480	L/513	Pass	7.0	2.7		
FB-6	15	2	3	80					1163	900	525	1163	900	525	8367	27.2	2231	18.8	(3) 1-3/4"x11-7/8" LVL	Live	0.11	L/480	L/1625	Pass	7.5	0.7		
FB-7	7		11.5						604	1610	0	604	1610	0	3874	47.4	2214	45.9	(2) 1-3/4"x7-1/4" LVL	Live	0.11	L/480	L/751	Pass	3.5	0.8		
FB-8	15		11.5						1294	3450	0	1294	3450	0	17789	57.8	4744	40.0	(3) 1-3/4"x11-7/8" LVL	Live	0.36	L/480	L/503	Pass	7.5	1.2		
FB-9	16	26	11.5						4500	3680	7280	4500	3680	7280	50880	94.8	12720	79.7	(3) 1-3/4"x16" LVL	Live	0.42	L/360	L/454	Pass	8.0	3.9		
FB-10	16		13.5						1620	4320	0	1620	4320	0	23760	56.8	5940	42.5	(3) 1-3/4"x14" LVL	Live	0.33	L/480	L/579	Pass	8.0	1.5		
FB-11	16	12	2						1680	640	3360	1680	640	3360	20160	48.2	5040	36.1	(3) 1-3/4"x14" LVL	Live	0.26	L/480	L/744	Pass	8.0	1.4		
FB-12	6	7	4	80					735	480	735	735	480	735	2469	60.5	1646	42.0	(3) 2x8	Live	0.04	L/480	L/1859	Pass	3.0	0.7		
FB-13	5		22						825	2200	0	825	2200	0	3781	92.7	3025	77.3	(3) 2x8	Live	0.05	L/480	L/1109	Pass	2.5	1.1		
FB-14	7		10.5						551	1470	0	551	1470	0	3537	86.7	2021	51.6	(3) 2x8	Live	0.10	L/480	L/846	Pass	3.5	0.7		
FB-15	7		5	200					963	700	0	963	700	0	2909	14.2	1663	21.1	(2) 1-3/4"x11-7/8" LVL	Live	0.01	L/480	L/7594	Pass	3.5	0.6		
FB-16	7		8	200	963	700	3.5		1182	350	0	1182	350	0	4135	20.1	1532	19.4	(2) 1-3/4"x11-7/8" LVL	Live	0.01	L/480	L/9493	Pass	3.5	0.6		
FB-17	5		22						825	2200	0	825	2200	0	3781	51.6	3025	55.1	(3) 1-3/4"x5-1/2" LVL	Live	0.09	L/480	L/706	Pass	2.5	0.8		
FB-18	13		8						780	2080	0	780	2080	0	9295	48.1	2860	38.5	3-1/2"x12" GLB	Live	0.23	L/480	L/688	Pass	6.5	1.3		
FB-19									0	0	0	0	0	0	0		0			Live		L/480						
FB-20									0	0	0	0	0	0	0		0			Live		L/480						
FB-21									0	0	0	0	0	0	0		0			Live		L/480						
FB-22									0	0	0	0	0	0	0		0			Live		L/480						
FB-23									0	0	0	0	0	0	0		0			Live		L/480						
FB-24									0	0	0	0	0	0	0		0			Live		L/480						
FB-25									0	0	0	0	0	0	0		0			Live		L/480						
FB-26									0	0	0	0	0	0	0		0			Live		L/480						
FB-27									0	0	0	0	0	0	0		0			Live		L/480						

FOOTINGS:

ALLOWABLE BEARING PRESSURE = 1.5 KSF  
 Assumed Coefficient of Friction Soil to Foundation = 0.3

Frame Line	GRAVITY										Required FTG	Specified FTG	
	DL	LL	SL	Trib <sub>DL</sub>	Trib <sub>LL</sub>	Trib <sub>SL</sub>	Wall	Trib <sub>Wall</sub>	DL	LL			
mid bear	15.0 psf	40.0 psf	35.0 psf	22.0ft	22.0ft	0.0ft	12.0 psf	25.0ft	0.6 k	0.9k	12.1 in	FC- 18	
mid rear	15.0 psf	40.0 psf	35.0 psf	39.0ft	16.0ft	23.0ft	40.0 psf	25.0ft	1.6 k	1.4k	24.2 in	FC- 30	
end rear	15.0 psf	40.0 psf	35.0 psf	21.0ft	18.0ft	3.0ft	40.0 psf	25.0ft	1.3 k	0.8k	17.1 in	FC- 20	
side	15.0 psf	40.0 psf	35.0 psf	20.0ft	4.0ft	16.0ft	40.0 psf	25.0ft	1.3 k	0.7k	16.2 in	FC- 20	
marriage	15.0 psf	40.0 psf	35.0 psf	21.0ft	4.0ft	17.0ft	15.0 psf	50.0ft	1.1 k	0.8k	14.6 in	FC- 20	
									0.0 k	0.0k	0.0 in	FC- 22	
	15.0 psf	40.0 psf	35.0 psf	33.0ft	28.0ft	21.0ft	51.0 psf	10.0ft	1.0 k	1.9k	22.9 in	FC- 24	
SPOT FTGS													
R porch	B	15.0 psf	40.0 psf	35.0 psf	315.0sf	0.0sf	195.0sf	0.0 psf	0.0sf	4.7 k	6.8k	33.3 in	FS- 36
F porch	C	15.0 psf	40.0 psf	35.0 psf	50.0sf	0.0sf	25.0sf	0.0 psf	0.0sf	0.8 k	0.9k	12.5 in	FS- 24
1	D	15.0 psf	0.0 psf	35.0 psf	0.0sf	0.0sf	0.0sf	0.0 psf	0.0sf	0.0 k	0.0k	0.0 in	FS- 0

TYPICAL FOOTINGS AND REINFORCING

Soil Bearing Pressure = 1500 psf      Footing Parameters      f<sub>c</sub> = 2500 psi      Note: Not all footings were used on this project  
 f<sub>y</sub> steel = 60 ksi  
 p<sub>max</sub> = 0.016

Continuous Footings						Trial Reinf				
Callout	Typ Eccent	Allowable Load	Ftg Req'd	Nom Ftg	Asmin/ft	As Req'd/ft	Num Bar	Bar #	As Prov	/ As
FC-1.5	0.1%	2.2 klf	1.48'	1.5'	0.18 in <sup>2</sup>	0.36 in <sup>2</sup>	(2)	4	0.4	OK
FC-2.0	0.1%	3.0 klf	1.98'	2.0'	0.24 in <sup>2</sup>	0.48 in <sup>2</sup>	(3)	4	0.6	OK
FC-2.5	0.1%	3.7 klf	2.48'	2.5'	0.30 in <sup>2</sup>	0.60 in <sup>2</sup>	(3)	5	0.93	OK
FC-3.0	0.1%	4.5 klf	2.98'	3.0'	0.36 in <sup>2</sup>	0.72 in <sup>2</sup>	(3)	5	0.93	OK
FC-3.5	0.1%	5.2 klf	3.48'	3.5'	0.42 in <sup>2</sup>	0.84 in <sup>2</sup>	(3)	5	0.93	OK
FC-4.0	0.1%	6.0 klf	3.98'	4.0'	0.48 in <sup>2</sup>	0.96 in <sup>2</sup>	(4)	5	1.24	OK
FC-4.5	0.1%	6.7 klf	4.48'	4.5'	0.54 in <sup>2</sup>	1.08 in <sup>2</sup>	(4)	5	1.24	OK
FC-5.0	0.1%	7.5 klf	4.98'	5.0'	0.60 in <sup>2</sup>	1.20 in <sup>2</sup>	(5)	5	1.55	OK
FTS-1.5	0.1%	2.2 klf	1.48'	1.5'	0.18 in <sup>2</sup>	0.36 in <sup>2</sup>	(2)	4	0.4	OK
FTS-2.0	0.1%	3.0 klf	1.98'	2.0'	0.24 in <sup>2</sup>	0.48 in <sup>2</sup>	(3)	4	0.6	OK
FTS-2.5	0.1%	3.7 klf	2.48'	2.5'	0.30 in <sup>2</sup>	0.60 in <sup>2</sup>	(3)	5	0.93	OK
FTS-3.0	0.1%	4.5 klf	2.98'	3.0'	0.36 in <sup>2</sup>	0.72 in <sup>2</sup>	(3)	5	0.93	OK
FTS-3.5	0.1%	5.2 klf	3.48'	3.5'	0.42 in <sup>2</sup>	0.84 in <sup>2</sup>	(3)	5	0.93	OK

Spot Footings							Trial Reinf						
Callout	Typ Eccent	Allowable Load	Ftg Req'd	Nom Ftg	Mu	p	/ p	Asmin	As Req'd	Num Bar	Bar #	As Prov	/ As
FS-2.0	0.1%	5.9 k	1.99'	2.0'	1.02 k-ft	0.000	OK	0.43 in <sup>2</sup>	0.05 in <sup>2</sup>	(2)	5	0.62	OK
FS-2.5	0.1%	9.2 k	2.49'	2.5'	2.16 k-ft	0.000	OK	0.54 in <sup>2</sup>	0.10 in <sup>2</sup>	(3)	5	0.93	OK
FS-3.0	0.1%	13.3 k	2.99'	3.0'	3.95 k-ft	0.001	OK	0.65 in <sup>2</sup>	0.18 in <sup>2</sup>	(3)	5	0.93	OK
FS-3.5	0.1%	18.2 k	3.49'	3.5'	6.52 k-ft	0.000	OK	0.90 in <sup>2</sup>	0.22 in <sup>2</sup>	(3)	5	0.93	OK
FS-4.0	0.1%	23.8 k	3.99'	4.0'	10.02 k-ft	0.001	OK	1.03 in <sup>2</sup>	0.33 in <sup>2</sup>	(4)	5	1.24	OK
FS-4.5	0.1%	30.1 k	4.49'	4.5'	14.58 k-ft	0.001	OK	1.16 in <sup>2</sup>	0.48 in <sup>2</sup>	(4)	5	1.24	OK
FS-5.0	0.1%	37.2 k	4.99'	5.0'	19.63 k-ft	0.001	OK	1.29 in <sup>2</sup>	0.65 in <sup>2</sup>	(5)	5	1.55	OK
FS-5.5	0.1%	45.0 k	5.50'	5.5'	26.59 k-ft	0.001	OK	1.42 in <sup>2</sup>	0.89 in <sup>2</sup>	(5)	5	1.55	OK
FS-6.0	0.1%	53.5 k	5.99'	6.0'	33.81 k-ft	0.001	OK	1.60 in <sup>2</sup>	1.07 in <sup>2</sup>	(6)	5	1.86	OK

PUNCHING SHEAR CHECK

Note: Not all footings were used on this project

Callout	Factored Load	Min Col. Dim	factored qu	Assumed depth	d	bo	Vu	Vc	/ Capacity
FS-2.0	8.5 k	4"	2.12 ksf	10"	7.0"	44"	6.7 k	52.4 k	OK
FS-2.5	13.3 k	4"	2.13 ksf	10"	7.0"	44"	11.5 k	52.4 k	OK
FS-3.0	19.2 k	4"	2.13 ksf	10"	7.0"	44"	17.4 k	52.4 k	OK
FS-3.5	26.2 k	4"	2.14 ksf	12"	9.0"	52"	23.7 k	79.6 k	OK
FS-4.0	34.2 k	4"	2.14 ksf	12"	9.0"	52"	31.7 k	79.6 k	OK
FS-4.5	43.4 k	4"	2.14 ksf	12"	9.0"	52"	40.8 k	79.6 k	OK
FS-5.0	53.6 k	5"	2.14 ksf	12"	9.0"	56"	50.7 k	85.7 k	OK
FS-5.5	64.9 k	5"	2.14 ksf	12"	9.0"	56"	61.9 k	85.7 k	OK
FS-6.0	77.0 k	6"	2.14 ksf	12"	9.4"	62"	73.5 k	98.4 k	OK



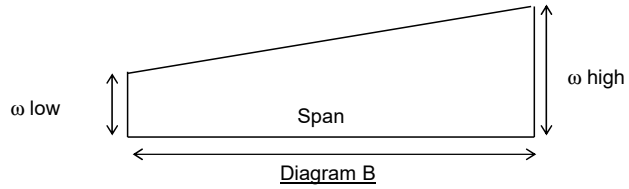
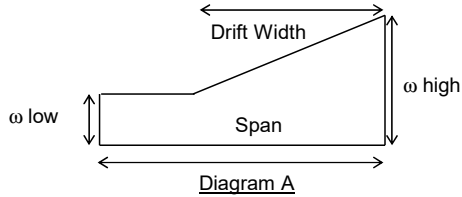


PROJECT			PROJ NO.	
			CALC NO.	
SUBJECT	<b>Low Roof Snow Load per ASCE 7 Leeward and Windward Snow Drift</b>		BY JMG	DATE 1/8/20
			CHK	DATE
			SHEET	OF REV

**Description:**

**GENERAL INPUT AND OUTPUT:**

"Leeward" or "Windward":	1.00	Leeward	Snow Load Density:	19.83 pcf
Roof Slope:	8:12	33.69 °	h <sub>b</sub> (Snow Depth):	1.74 feet
County (State of Utah)	UTAH		h <sub>d</sub> (potential drift height):	-1.50 feet
Elevation at Site	4700 ft		h <sub>c</sub> (Roof to Snow):	0.00 feet
p <sub>g</sub> (SEAU Flat Roof Snow)	45 psf	<---Manual Entry	Maximum Drift Height:	0.00 feet
C <sub>e</sub> (Exposure Factor):	1.0		Drift Width:	0.00 feet
C <sub>t</sub> (Thermal Factor):	1.1		Maximum Snow Load:	<b>34.50 psf</b>
I <sub>s</sub> (Importance Factor):	1.0		Maximum Drift Weight:	<b>0.00 psf</b>
L <sub>u</sub> (Length of Roof):	0.00 feet	Upper Roof	Drift Width:	<b>0.00 feet</b>
Elevation Difference:	0.00 feet			
P <sub>f</sub> (Roof Snow):	<b>34.5 psf</b>			
C <sub>s</sub> (Roof Slope Factor):	1.0			
P <sub>s</sub> (Sloped Roof Snow):	<b>34.5 psf</b>			



	<h2 style="margin: 0;">Reeve &amp; Associates, Inc.</h2> <p style="margin: 0; font-size: small;">Land Planners · Civil Engineers · Land Surveyors Traffic Engineers · Structural Engineers · Landscape Architects 5160 South 1500 West - Riverdale, Utah 84405 Phone: (801) 621-3100 Fax: (801) 621-2666</p>			
	Date: <b>1/8/2020</b>	Sheet 1	Of 1	
	Designed By: <b>JMG</b>	Project Number:		

### Seismic Calculations

#### Earthquake Loads-Site Ground Motion

I= 1	h <sub>n</sub> = 28.00 ft (Building Height)	
R= 6.5 Wood Brg Wall	Structure Type = Other	
S <sub>s</sub> = 1.139	C <sub>t</sub> = 0.02	
S <sub>1</sub> = 0.385	x= 0.75	
Site Class= D		
	Check Height for LFRS = 28.0ft > NP (per exception 12.2.5.6 where	
	DL = 6.5 psf (Estimated) DL < 20psf allowed up to 65ft)	
F <sub>a</sub> = 1.04	S <sub>MS</sub> = F <sub>a</sub> *S <sub>s</sub>	
S <sub>MS</sub> = 1.190	S <sub>DS</sub> = 2*S <sub>MS</sub> /3	
S <sub>DS</sub> = 0.793		
F <sub>v</sub> = 1.63	S <sub>M1</sub> = F <sub>v</sub> *S <sub>1</sub>	
S <sub>M1</sub> = 0.628	S <sub>D1</sub> = 2*S <sub>M1</sub> /3	
S <sub>D1</sub> = 0.418		

#### Earthquake Loads-Minimum Design Lateral Force

C <sub>s</sub> = 0.122	T= 0.243 s	
C <sub>s</sub> = 0.264		Load Combinations that control lateral:
C <sub>s</sub> = 0.035		D+0.75L+0.75S+0.75(0.7*E)
	ASD Load Factor = 0.7	0.6D+0.7E
USE	Rho = 1.3	
C <sub>s</sub> = 0.122		
V=C <sub>s</sub> *W	V = ASD Load Factor*Rho*C <sub>s</sub> *W = 0.11 *W	

#### Dead Load Effect

$$0.2 * S_{DS} = 0.159$$

$$(+/-) 0.2 * S_{DS} * D$$

#### Seismic Design Category

SDS => D

SD1 => D

Wind Design - ASCE 7-16 Chp 27

Risk Category = II  
 Basic Wind Speed V = 115 mph  
 Exposure Category = C 3  
 Wind Directionality Factor, K<sub>d</sub> = 0.85  
 Topographic Factor, K<sub>t</sub> = 1  
 Gust Effect Factor, G = 0.85  
 Total Stories = 2 (5 max)  
 Internal Pressure Coefficient, G<sub>cpi</sub> = 0.18  
 -0.18

Enclosure Classification				Ag	Ao	% open	Open	Partial 1	Partial 2	Partial Total
Length	Height									
Wall 1 =	100	10	1000	800	80.0	Y	Y	N	N	N
Wall 2 =	100	10	1000	200	20.0	N	N	Y	N	N
Wall 3 =	25	10	250	30	12.0	N	N	Y	N	N
Wall 4 =	25	10	250	50	20.0	N	N	Y	N	N

Open Building: NO  
 Partially Open: NO  
 Enclosed: YES

Wall Pressure	Front to Back	Side to Side
Windward Wall, C <sub>p</sub> =	0.8	0.8
Windward Wall Width, B =	28 ft	48 ft
Exposure Category =	C	C
Side Wall Width, L =	48 ft	28 ft
L/B =	1.714286	0.583333
Leeward Wall, C <sub>p</sub> =	-0.3	-0.5
Side Wall, C <sub>p</sub> =	-0.7	-0.7

Parapet Wall Pressure  
 Parapet Wall Height = 0 ft  
 Building Height to top of parapet = 0 ft  
 K<sub>z</sub> = 0.85  
 q<sub>p</sub> = 24.4 psf  
 WW GC<sub>p</sub> = 1.50  
 LW GC<sub>p</sub> = -1.00  
 P<sub>p</sub> = 61.1 psf  
 Adj. P<sub>p</sub> = 36.6 psf  
 Parapet Load per foot = 0.0 plf

Roof Pressure	Gable 1	Gable 2
Roof Type =	Gable 1	Gable 2
Roof Pitch =	8/12 33.7 °	8/12 33.7 °
Ridge Height =	28 ft	28 ft
Eave Height =	18 ft	18 ft
Mean Roof Height, h =	22.8 ft	22.8 ft
h/L =	0.47	0.81
h/2 =	11 ft	11 ft
Kh =	0.93	0.93
qh =	26.7 psf	26.7 psf

Roof Pressure Coefficient, C <sub>p</sub>	WW Area: 480 ft <sup>2</sup>   LW Area: 480 ft <sup>2</sup>				WW Area: 228 ft <sup>2</sup>   LW Area: 228 ft <sup>2</sup>					
	Front to Back				Side to Side					
	Distance from Windward Edge, ft				Distance from Windward Edge, ft					
Windward Normal to Ridge =	Max/Min	0 ft	11 ft	23 ft	46 ft	Max/Min	0 ft	11 ft	23 ft	46 ft
	Max	0.28	0.28	0.28	0.28	Max	0.23	0.23	0.23	0.23
	Min	-0.18	-0.18	-0.18	-0.18	Min	-0.22	-0.22	-0.22	-0.22
Leeward Normal to Ridge =	Max	-0.60	-0.60	-0.60	-0.60	Max	-0.60	-0.60	-0.60	-0.60
	Min	-0.60	-0.60	-0.60	-0.60	Min	-0.60	-0.60	-0.60	-0.60
Parallel to Ridge =	Max	-0.18	-0.18	-0.18	-0.18	Max	-0.18	-0.18	-0.18	-0.18
	Min	-0.90	-0.90	-0.50	-0.30	Min	-1.15	-1.15	-0.63	-0.55

Load Description	Overall Height	Wall Trib	K <sub>z</sub>	q <sub>z</sub>	Front to Back				Total	Shear Force, lbs	Adj. Shear Force, lbs	ASD Factor	
					WW	LW	SW	Int +/-					
Roof	22.8 ft	-	-	max-->	6.4 psf	-13.6 psf	varies	4.8 psf	20.0 psf	5331 lbs	3199 lbs	0.6	
				min-->	-4.2 psf	-13.6 psf	varies	-4.8 psf	9.4 psf	2505 lbs	1503 lbs		
Wall 2	8 ft	4.5	0.85		24.4 psf	16.6 psf	-6.8 psf	-15.9 psf	4.8 psf	23.4 psf	2950 lbs	1770 lbs	
Wall 1	9 ft	8.5	0.85		24.4 psf	16.6 psf	-6.8 psf	-15.9 psf	4.8 psf	23.4 psf	5572 lbs	3343 lbs	
			0.85		24.4 psf	16.6 psf	-6.8 psf	-15.9 psf	4.8 psf	23.4 psf	0 lbs	0 lbs	
			0.85		24.4 psf	16.6 psf	-6.8 psf	-15.9 psf	4.8 psf	23.4 psf	0 lbs	0 lbs	
Other			0.85		24.4 psf	16.6 psf	-6.8 psf	-15.9 psf	4.8 psf	23.4 psf	0 lbs	0 lbs	

Front to Back Total Shear: 13853 lbs 8312 lbs

Roof	max-->	Side to Side				Total	Shear Force, lbs	Adj. Shear Force, lbs	Adj. Wall Force
		WW	LW	SW	Int +/-				
		5.2 psf	-13.6 psf	varies	4.8 psf	18.8 psf	2373 lbs	1424 lbs	3823 lbs
		-4.9 psf	-13.6 psf	varies	-4.8 psf	8.7 psf	1100 lbs	660 lbs	
Wall 2		16.6 psf	-11.3 psf	-15.9 psf	4.8 psf	27.9 psf	6036 lbs	3622 lbs	
Wall 1		16.6 psf	-11.3 psf	-15.9 psf	4.8 psf	27.9 psf	11402 lbs	6841 lbs	
		16.6 psf	-11.3 psf	-15.9 psf	4.8 psf	27.9 psf	0 lbs	0 lbs	
		16.6 psf	-11.3 psf	-15.9 psf	4.8 psf	27.9 psf	0 lbs	0 lbs	
Other		16.6 psf	-11.3 psf	-15.9 psf	4.8 psf	27.9 psf	0 lbs	0 lbs	

Side to Side Total Shear: 19811 lbs 14286 lbs

**LATERAL ANALYSIS**

Side-to-Side Dim:	28 ft	Height	Roof Area =	1344 ft <sup>2</sup>	
Front-to-Back Dim:	48 ft		Floor Area =	1344	
Roof Trib:	4.5 ft		18 ft	Floor Area =	
Floor Trib:	8.5 ft		9 ft	Floor Area =	
Floor Trib:				Floor Area =	
Floor Trib:					
Roof Seismic DL:	15 psf				
Floor Seismic DL:	15 psf				
Wall Seismic DL:	12 psf				

**Seismic:**                      V =    0.11    \*W

F-front-to-back:	<u>V</u>		<u>W</u>	<u>WxHx</u>	<u>Cvx</u>	<u>Fx</u>
Roof	2574 lbs	lb	23184	417312	0.64	3496 lbs
Floor	2872 lbs	lb	25872	232848	0.36	1951 lbs
Floor	0 lbs	lb	0	0	0.00	0 lbs
Floor	0 lbs	lb	0	0	0.00	0 lbs
	5447 lbs		49056	650160		5447 lbs
<hr/>						
F-side-to-side:						
Roof	2814 lbs	lb	25344	456192	0.63	3859 lbs
Floor	3325 lbs	lb	29952	269568	0.37	2280 lbs
Floor	0 lbs	lb	0	0	0.00	0 lbs
Floor	0 lbs	lb	0	0	0.00	0 lbs
	6139 lbs		55296	725760		6139 lbs

**Wind:**

F-front-to-back:	
Roof	4969 lbs
Floor	3343 lbs
Floor	0 lbs
Floor	0 lbs
F-side-to-side:	
Roof	7445 lbs
Floor	6841 lbs
Floor	0 lbs
Floor	0 lbs

**Use for Design:**

F-front-to-back:		
Roof	4969 lbs	<b>Wind Governs</b> 1.4
Floor	3343 lbs	<b>Wind Governs</b> 1.4
Floor	0 lbs	1
Floor	0 lbs	1
F-side-to-side:		
Roof	7445 lbs	<b>Wind Governs</b> 1.4
Floor	6841 lbs	<b>Wind Governs</b> 1.4
Floor	0 lbs	1
Floor	0 lbs	1

	SW capacities (plf):		Hold Down capacities (lb):			
	seismic	wind	0	NONE	0	NONE
SW-1	260	365	200	LSTD8    1610	200	MST37    1725
SW-2	380	532	1610	STHD10   2175	1725	MST48    3215
SW-3	490	685	2175	STHD14   3500	3215	MST60    5240
SW-4	640	896	3500	HDU4      4565	5240	MST72    6730
SW-5	760	1065	4565	HDU5      5645	6730	(2) MST60 10480
SW-6	980	1370	5645	HDU8      7870	10480	(2) MST72 13460
			7870	HDU11     9535	13460	NG















Simple Span Beam Calculation

Adjustment Factors - ASD

Cd:	1.15	Cfu:	NA
Cm:	1.00	Ci:	1.00
Ct:	1.00	Cr:	1.00
Ci:	1.00	Cv:	-
Cf:	-	Cc:	1.00

Glu-Lam & LVL Only  
Glu-Lam Only

Roof DL	10	psf	Include Self Wt?
Floor DL	15	psf	No
Live	40	psf	
Snow	35	psf	

---> Point Load distance must be >= midspan distance!

Active Member for RB-5  
Deflection Calc and Shear Diagram:

member ID	span ft	roof trib ft	floor trib ft	wall load plf	point load			dist from left, ft	left reaction				right reaction				applied moment lb-ft	% Str	applied shear lbs	% Str	Selected Member	Live/Total Deflection	actual Δ, in.	Δ limit, L/	Δ act, L/	check	Max Defl Location, ft	Min Bearing Length, in
					dead lbs	live lbs	snow lbs		dead lbs	live lbs	snow lbs	dead lbs	live lbs	snow lbs	dead lbs	live lbs												
RB-1	4	16						320	0	1120	320	0	1120	1440	52.9	1440	48.0	(2) 2x8	Live	0.02	L/240	L/2268	Pass	2.0	0.8			
RB-2	6	5						150	0	525	150	0	525	1013	37.2	675	22.5	(2) 2x8	Live	0.03	L/240	L/2151	Pass	3.0	0.4			
RB-3	6	26						780	0	2730	780	0	2730	5265	64.4	3510	63.3	(2) 1-3/4"x7-1/4" LVL	Live	0.12	L/240	L/603	Pass	3.0	1.3			
RB-4	8	8						320	0	1120	320	0	1120	2880	70.6	1440	32.0	(3) 2x8	Live	0.11	L/240	L/851	Pass	4.0	0.5			
RB-5	3	27						405	0	1418	405	0	1418	1367	50.3	1823	60.7	(2) 2x8	Live	0.01	L/360	L/3186	Pass	1.5	1.0			
RB-6								0	0	0	0	0	0	0		0			Live									
RB-7								0	0	0	0	0	0	0		0			Live									
RB-8								0	0	0	0	0	0	0		0			Live									
RB-9								0	0	0	0	0	0	0		0			Live									
RB-10								0	0	0	0	0	0	0		0			Live									
RB-11								0	0	0	0	0	0	0		0			Live									
RB-12								0	0	0	0	0	0	0		0			Live									
RB-13								0	0	0	0	0	0	0		0			Live									
RB-14								0	0	0	0	0	0	0		0			Live									
RB-15								0	0	0	0	0	0	0		0			Live									
RB-16								0	0	0	0	0	0	0		0			Live									
RB-17								0	0	0	0	0	0	0		0			Live									
RB-18								0	0	0	0	0	0	0		0			Live									
RB-19								0	0	0	0	0	0	0		0			Live									
RB-20								0	0	0	0	0	0	0		0			Live									
RB-21								0	0	0	0	0	0	0		0			Live									
RB-22								0	0	0	0	0	0	0		0			Live									
RB-23								0	0	0	0	0	0	0		0			Live									
RB-24								0	0	0	0	0	0	0		0			Live									
RB-25								0	0	0	0	0	0	0		0			Live									
RB-26								0	0	0	0	0	0	0		0			Live									
RB-27								0	0	0	0	0	0	0		0			Live									

Simple Span Beam Calculation

Adjustment Factors - ASD

Cd:	1.00	Cfu:	NA
Cm:	1.00	Ci:	1.00
Ct:	1.00	Cr:	1.15
Ci:	1.00	Cv:	-
Cf:	-	Cc:	1.00

Glu-Lam & LVL Only  
Glu-Lam Only

Roof DL	15	psf	Include Self Wt?
Floor DL	15	psf	No
Live	40	psf	
Snow	35	psf	

---> Point Load distance must be >= midspan distance!

Active Member for Deflection Calc and Shear Diagram: FB-18

member ID	span ft	roof trib ft	floor trib ft	wall load plf	point load			dist from left, ft	left reaction				right reaction				applied moment lb-ft	% Str	applied shear lbs	% Str	Selected Member	Live/Total Deflection	actual Δ, in.	Δ limit L/	Δ act L/	check	Max Defl Location, ft	Min Bearing Length, in
					dead lbs	live lbs	snow lbs		dead lbs	live lbs	snow lbs	dead lbs	live lbs	snow lbs														
FB-1	4		16						480	1280	0	480	1280	0	1760	64.7	1760	67.4	(2) 2x8	Live	0.04	L/360	L/1227	Pass	3.2	0.9		
FB-2	6		13						585	1560	0	585	1560	0	3218	78.9	2145	54.8	(3) 2x8	Live	0.07	L/360	L/1086	Pass	3.0	0.8		
FB-3	6		13						585	1560	0	585	1560	0	3218	79.3	2145	64.4	(2) 2x10	Live	0.05	L/360	L/1503	Pass	3.0	1.1		
FB-4	4		7						210	560	0	210	560	0	770	7.5	770	19.5	1-3/4"x11-7/8" LVL	Live	0.00	L/480	L/14536	Pass	2.0	0.6		
FB-5	14	26	3	80					3605	840	6370	3605	840	6370	34913	83.4	9975	71.4	(3) 1-3/4"x14" LVL	Live	0.33	L/480	L/513	Pass	7.0	2.7		
FB-6	15	2	3	80					1163	900	525	1163	900	525	8367	27.2	2231	18.8	(3) 1-3/4"x11-7/8" LVL	Live	0.11	L/480	L/1625	Pass	7.5	0.7		
FB-7	7		11.5						604	1610	0	604	1610	0	3874	47.4	2214	45.9	(2) 1-3/4"x7-1/4" LVL	Live	0.11	L/480	L/751	Pass	3.5	0.8		
FB-8	15		11.5						1294	3450	0	1294	3450	0	17789	57.8	4744	40.0	(3) 1-3/4"x11-7/8" LVL	Live	0.36	L/480	L/503	Pass	7.5	1.2		
FB-9	16	26	11.5						4500	3680	7280	4500	3680	7280	50880	94.8	12720	79.7	(3) 1-3/4"x16" LVL	Live	0.42	L/360	L/454	Pass	8.0	3.9		
FB-10	16		13.5						1620	4320	0	1620	4320	0	23760	56.8	5940	42.5	(3) 1-3/4"x14" LVL	Live	0.33	L/480	L/579	Pass	8.0	1.5		
FB-11	16	12	2						1680	640	3360	1680	640	3360	20160	48.2	5040	36.1	(3) 1-3/4"x14" LVL	Live	0.26	L/480	L/744	Pass	8.0	1.4		
FB-12	6	7	4	80					735	480	735	735	480	735	2469	60.5	1646	42.0	(3) 2x8	Live	0.04	L/480	L/1859	Pass	3.0	0.7		
FB-13	5		22						825	2200	0	825	2200	0	3781	92.7	3025	77.3	(3) 2x8	Live	0.05	L/480	L/1109	Pass	2.5	1.1		
FB-14	7		10.5						551	1470	0	551	1470	0	3537	86.7	2021	51.6	(3) 2x8	Live	0.10	L/480	L/846	Pass	3.5	0.7		
FB-15	7		5	200					963	700	0	963	700	0	2909	14.2	1663	21.1	(2) 1-3/4"x11-7/8" LVL	Live	0.01	L/480	L/7594	Pass	3.5	0.6		
FB-16	7		8	200	963	700	3.5		1182	350	0	1182	350	0	4135	20.1	1532	19.4	(2) 1-3/4"x11-7/8" LVL	Live	0.01	L/480	L/9493	Pass	3.5	0.6		
FB-17	5		22						825	2200	0	825	2200	0	3781	51.6	3025	55.1	(3) 1-3/4"x5-1/2" LVL	Live	0.09	L/480	L/706	Pass	2.5	0.8		
FB-18	13		8						780	2080	0	780	2080	0	9295	53.9	2860	43.2	3-1/8"x12" GLB	Live	0.25	L/480	L/614	Pass	6.5	1.4		
FB-19									0	0	0	0	0	0	0		0			Live		L/480						
FB-20									0	0	0	0	0	0	0		0			Live		L/480						
FB-21									0	0	0	0	0	0	0		0			Live		L/480						
FB-22									0	0	0	0	0	0	0		0			Live		L/480						
FB-23									0	0	0	0	0	0	0		0			Live		L/480						
FB-24									0	0	0	0	0	0	0		0			Live		L/480						
FB-25									0	0	0	0	0	0	0		0			Live		L/480						
FB-26									0	0	0	0	0	0	0		0			Live		L/480						
FB-27									0	0	0	0	0	0	0		0			Live		L/480						

FOOTINGS:

ALLOWABLE BEARING PRESSURE = 1.5 KSF  
 Assumed Coefficient of Friction Soil to Foundation = 0.3

Frame Line	GRAVITY										Required FTG	Specified FTG	
	DL	LL	SL	Trib <sub>DL</sub>	Trib <sub>LL</sub>	Trib <sub>SL</sub>	Wall	Trib <sub>Wall</sub>	DL	LL			
mid bear	15.0 psf	40.0 psf	35.0 psf	22.0ft	22.0ft	0.0ft	12.0 psf	25.0ft	0.6 k	0.9k	12.1 in	FC- 18	
mid rear	15.0 psf	40.0 psf	35.0 psf	39.0ft	16.0ft	23.0ft	40.0 psf	25.0ft	1.6 k	1.4k	24.2 in	FC- 30	
end rear	15.0 psf	40.0 psf	35.0 psf	21.0ft	18.0ft	3.0ft	40.0 psf	25.0ft	1.3 k	0.8k	17.1 in	FC- 20	
side	15.0 psf	40.0 psf	35.0 psf	20.0ft	4.0ft	16.0ft	40.0 psf	25.0ft	1.3 k	0.7k	16.2 in	FC- 20	
marriage	15.0 psf	40.0 psf	35.0 psf	21.0ft	4.0ft	17.0ft	15.0 psf	50.0ft	1.1 k	0.8k	14.6 in	FC- 20	
									0.0 k	0.0k	0.0 in	FC- 22	
	15.0 psf	40.0 psf	35.0 psf	33.0ft	28.0ft	21.0ft	51.0 psf	10.0ft	1.0 k	1.9k	22.9 in	FC- 24	
SPOT FTGS													
R porch	B	15.0 psf	40.0 psf	35.0 psf	315.0sf	0.0sf	195.0sf	0.0 psf	0.0sf	4.7 k	6.8k	33.3 in	FS- 36
F porch	C	15.0 psf	40.0 psf	35.0 psf	50.0sf	0.0sf	25.0sf	0.0 psf	0.0sf	0.8 k	0.9k	12.5 in	FS- 24
1	D	15.0 psf	0.0 psf	35.0 psf	0.0sf	0.0sf	0.0sf	0.0 psf	0.0sf	0.0 k	0.0k	0.0 in	FS- 0

TYPICAL FOOTINGS AND REINFORCING

Soil Bearing Pressure = 1500 psf      Footing Parameters      f<sub>c</sub> = 2500 psi      Note: Not all footings were used on this project  
 f<sub>y</sub> steel = 60 ksi  
 p<sub>max</sub> = 0.016

Continuous Footings						Trial Reinf				
Callout	Typ Eccent	Allowable Load	Ftg Req'd	Nom Ftg	Asmin/ft	As Req'd/ft	Num Bar	Bar #	As Prov	/ As
FC-1.5	0.1%	2.2 klf	1.48'	1.5'	0.18 in <sup>2</sup>	0.36 in <sup>2</sup>	(2)	4	0.4	OK
FC-2.0	0.1%	3.0 klf	1.98'	2.0'	0.24 in <sup>2</sup>	0.48 in <sup>2</sup>	(3)	4	0.6	OK
FC-2.5	0.1%	3.7 klf	2.48'	2.5'	0.30 in <sup>2</sup>	0.60 in <sup>2</sup>	(3)	5	0.93	OK
FC-3.0	0.1%	4.5 klf	2.98'	3.0'	0.36 in <sup>2</sup>	0.72 in <sup>2</sup>	(3)	5	0.93	OK
FC-3.5	0.1%	5.2 klf	3.48'	3.5'	0.42 in <sup>2</sup>	0.84 in <sup>2</sup>	(3)	5	0.93	OK
FC-4.0	0.1%	6.0 klf	3.98'	4.0'	0.48 in <sup>2</sup>	0.96 in <sup>2</sup>	(4)	5	1.24	OK
FC-4.5	0.1%	6.7 klf	4.48'	4.5'	0.54 in <sup>2</sup>	1.08 in <sup>2</sup>	(4)	5	1.24	OK
FC-5.0	0.1%	7.5 klf	4.98'	5.0'	0.60 in <sup>2</sup>	1.20 in <sup>2</sup>	(5)	5	1.55	OK
FTS-1.5	0.1%	2.2 klf	1.48'	1.5'	0.18 in <sup>2</sup>	0.36 in <sup>2</sup>	(2)	4	0.4	OK
FTS-2.0	0.1%	3.0 klf	1.98'	2.0'	0.24 in <sup>2</sup>	0.48 in <sup>2</sup>	(3)	4	0.6	OK
FTS-2.5	0.1%	3.7 klf	2.48'	2.5'	0.30 in <sup>2</sup>	0.60 in <sup>2</sup>	(3)	5	0.93	OK
FTS-3.0	0.1%	4.5 klf	2.98'	3.0'	0.36 in <sup>2</sup>	0.72 in <sup>2</sup>	(3)	5	0.93	OK
FTS-3.5	0.1%	5.2 klf	3.48'	3.5'	0.42 in <sup>2</sup>	0.84 in <sup>2</sup>	(3)	5	0.93	OK

Spot Footings							Trial Reinf						
Callout	Typ Eccent	Allowable Load	Ftg Req'd	Nom Ftg	Mu	p	/ p	Asmin	As Req'd	Num Bar	Bar #	As Prov	/ As
FS-2.0	0.1%	5.9 k	1.99'	2.0'	1.02 k-ft	0.000	OK	0.43 in <sup>2</sup>	0.05 in <sup>2</sup>	(2)	5	0.62	OK
FS-2.5	0.1%	9.2 k	2.49'	2.5'	2.16 k-ft	0.000	OK	0.54 in <sup>2</sup>	0.10 in <sup>2</sup>	(3)	5	0.93	OK
FS-3.0	0.1%	13.3 k	2.99'	3.0'	3.95 k-ft	0.001	OK	0.65 in <sup>2</sup>	0.18 in <sup>2</sup>	(3)	5	0.93	OK
FS-3.5	0.1%	18.2 k	3.49'	3.5'	6.52 k-ft	0.000	OK	0.90 in <sup>2</sup>	0.22 in <sup>2</sup>	(3)	5	0.93	OK
FS-4.0	0.1%	23.8 k	3.99'	4.0'	10.02 k-ft	0.001	OK	1.03 in <sup>2</sup>	0.33 in <sup>2</sup>	(4)	5	1.24	OK
FS-4.5	0.1%	30.1 k	4.49'	4.5'	14.58 k-ft	0.001	OK	1.16 in <sup>2</sup>	0.48 in <sup>2</sup>	(4)	5	1.24	OK
FS-5.0	0.1%	37.2 k	4.99'	5.0'	19.63 k-ft	0.001	OK	1.29 in <sup>2</sup>	0.65 in <sup>2</sup>	(5)	5	1.55	OK
FS-5.5	0.1%	45.0 k	5.50'	5.5'	26.59 k-ft	0.001	OK	1.42 in <sup>2</sup>	0.89 in <sup>2</sup>	(5)	5	1.55	OK
FS-6.0	0.1%	53.5 k	5.99'	6.0'	33.81 k-ft	0.001	OK	1.60 in <sup>2</sup>	1.07 in <sup>2</sup>	(6)	5	1.86	OK

PUNCHING SHEAR CHECK

Note: Not all footings were used on this project

Callout	Factored Load	Min Col. Dim	factored qu	Assumed depth	d	bo	Vu	Vc	/ Capacity
FS-2.0	8.5 k	4"	2.12 ksf	10"	7.0"	44"	6.7 k	52.4 k	OK
FS-2.5	13.3 k	4"	2.13 ksf	10"	7.0"	44"	11.5 k	52.4 k	OK
FS-3.0	19.2 k	4"	2.13 ksf	10"	7.0"	44"	17.4 k	52.4 k	OK
FS-3.5	26.2 k	4"	2.14 ksf	12"	9.0"	52"	23.7 k	79.6 k	OK
FS-4.0	34.2 k	4"	2.14 ksf	12"	9.0"	52"	31.7 k	79.6 k	OK
FS-4.5	43.4 k	4"	2.14 ksf	12"	9.0"	52"	40.8 k	79.6 k	OK
FS-5.0	53.6 k	5"	2.14 ksf	12"	9.0"	56"	50.7 k	85.7 k	OK
FS-5.5	64.9 k	5"	2.14 ksf	12"	9.0"	56"	61.9 k	85.7 k	OK
FS-6.0	77.0 k	6"	2.14 ksf	12"	9.4"	62"	73.5 k	98.4 k	OK



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